

Foundation Design Updates for Ancillary Traffic Structures

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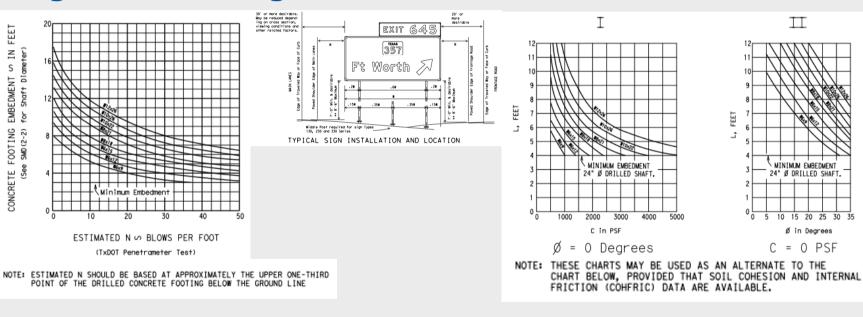
| Structure | Standard | Soil Strength Parameters | Design Loading Conditions |
|--|---------------------------|---------------------------------|-----------------------------------|
| Large Road Signs | SMD (8W2) - 08 | N _{TCP} , <i>C</i> , φ | Not Specified |
| Overhead Sign Bridge | OSB - FD OSB - FD - SC | N _{TCP} , <i>C</i> , φ | Uplift Loading and Moment |
| Monotube Sign Structure (Cantilever) | MC(7) - 22 | N _{TCP} | Axial, Moment, Torsion, and Shear |
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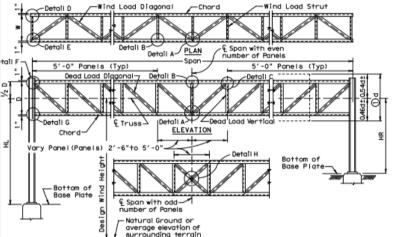
Large Roadside Signs

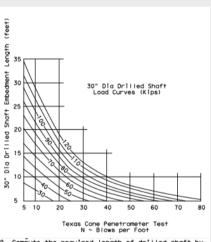




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Overhead Sign Bridges





 Compute the required length of drilled shaft by adding 3'-0" to the required embedment length.

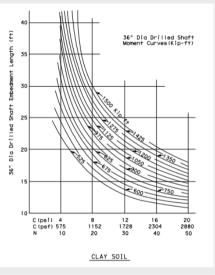
GENERAL NOTES:

These charts are to be used for Simple Span Overhead Sign Bridges with two shafts per tower. Numbers shown on curved lines are uplift in kip. Dead load of concrete in drilled shafts is included in curves.

Minimum embedment of drilled shafts is two diameters.

diameters.

Load curves shall not be extrapolated below
the N value of 5 blows per foot.



GENERAL NOTES:

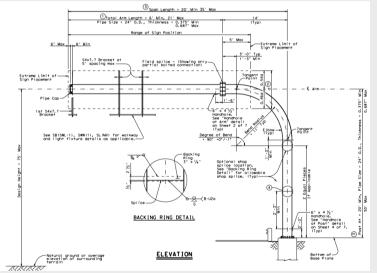
Minimum embedment of drilled shofts is two diameters. Add 3'-0" to required embedment length to determine required length of drilled shaft. These graphs are intended for use with Overhead Sign Bridges, with one shaft footing. (Not suitable for cantilever structures).

C = Cohesive shear strength of soil, in psf.



| Structure | Standard | Soil Strength Parameters | Design Loading Conditions |
|---|---------------------------|---------------------------------|-----------------------------------|
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Monotube Sign Structure - Cantilever



| TOP | TOP OF DRILLED SHAFT DESIGN LOADS | | | | | | |
|----------------|-----------------------------------|-----------------|------------------|-------------------|-----------------|--|--|
| | 90 | MPH W | IND SF | PEED | | | |
| Span Length | Post Height | Axial (kips) | Moment (k-ft) | Torsion (k-ft) | Shear (kips) | | |
| | 20' | 9 | 138 | 77 | 6 | | |
| | 25' | 9 | 166 | 77 | 6 | | |
| 201 | 30' | 10 | 195 | 77 | 7 | | |
| | 40' | 13 | 256 | 77 | 7 | | |
| | 50' | 19 | 321 | 77 | 7 | | |
| | 20, | 13 | 189 | 121 | 8 | | |
| | 25' | 14 | 225 | 121 | 8 | | |
| 25' | 30' | 15 | 262 | 121 | 8 | | |
| | 40' | 18 | 340 | 121 | 9 | | |
| | 50' | 16 | 418 | 121 | 8 | | |

| DRI | DRILLED SHAFT EMBEDMENT LENGTHS | | | | | | |
|----------------|---------------------------------|-------------------------|-------------------------|-------------------------|-------------------------|--|--|
| | 9 | 0 MPH | WIND S | PEED | | | |
| Span Length | Post Height | TCP 10 bl/ft (ft) | TCP 20 b1/ft (ft) | TCP 30 bl/ft (ft) | TCP 40+b1/ft (ft) | | |
| | 20' | 35 | 31 | 29 | 27 | | |
| 1 | 25' | 36 | 31 | 29 | 27 | | |
| 201 | 30' | 37 | 31 | 29 | 27 | | |
| 1 | 40' | 38 | 31 | 29 | 27 | | |
| | 50' | 40 | 31 | 29 | 27 | | |
| | 20' | 37 | 31 | 29 | 27 | | |
| 1 | 25' | 38 | 31 | 29 | 27 | | |
| 251 | 30' | 39 | 31 | 29 | 27 | | |
| | 40' | 41 | 31 | 29 | 27 | | |
| | 50' | 42 | 31 | 29 | 27 | | |

Determine foundation embedment length based on the blow counts in the upper 20 ft of soil.

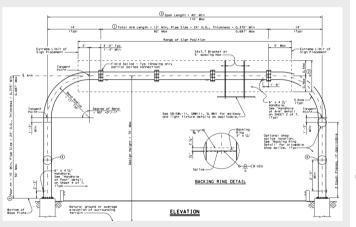
Terminate shafts encountering rock with a minimum rock penetration of 13 ft, while maintaining a minimum shaft embedment length of 25 ft.

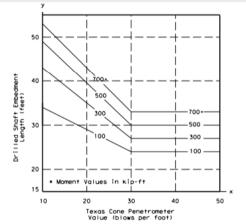
For Texas Cone Penetrometer (TCP) blow count data that falls between two of the listed values in the tables, it is permissible to use linear interpolation between the two nearest blow count values to determine foundation embedment length.



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|---|---------------------------|---------------------------------|-----------------------------------|
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Monotube Sign Structure - Span





48" Digmeter Drilled Shaft Load Curves (kip-ft)

Note: The Drilled Shoft Load Curves were developed in terms of applied moment, shear, and torsion. Moment combined with compart considered. While moment is used to represent the design curves in the Drilled Shoft Load Curve plot, shear was also taken into account.

The 700+ curve is valid for designing foundations for load cases with a moment greater than or equal to 700 k-ft.

Use the foundation design curves only for the design loading listed on this standard. A custom foundation design is required for soil profiles with blow counts less than 10 bl/ft.

Interpolate moment values that are between two curves.

| TOP OF DRILLED SHAFT DESIGN LOADS | | | | | | |
|-----------------------------------|----------------|-----------------|------------------|-------------------|-----------------|--|
| | 90 | MPH W | IND SF | PEED | | |
| Span Length | Post Height | Axial (kips) | Moment (k-ft) | Torsion (k-f+) | Shear (kips) | |
| | 15′ | 7 | 119 | 33 | 11 | |
| | 20' | 8 | 134 | 27 | 9 | |
| 401 | 30' | 10 | 189 | 24 | 8 | |
| | 40' | 14 | 251 | 22 | 7 | |
| | 50' | 20 | 316 | 23 | 8 | |
| | 15' | 8 | 150 | 51 | 14 | |
| | 20' | 9 | 169 | 44 | 11 | |
| 501 | 30' | 12 | 237 | 41 | 9 | |
| | 40' | 18 | 314 | 44 | 9 | |
| | 50' | 13 | 389 | 25 | 9 | |

Determine foundation embedment length based on the blow counts in the upper 20 ft of soil.

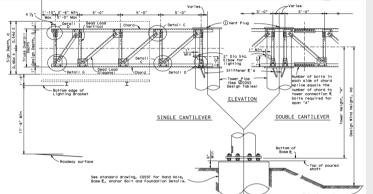
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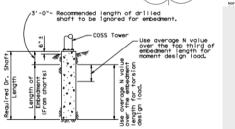


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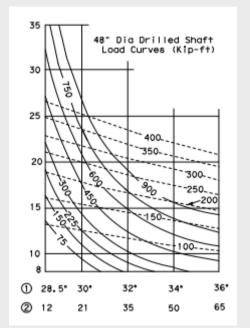


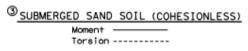
Cantilever Overhead Sign Support

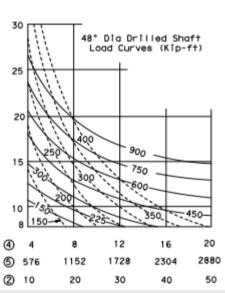


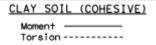


- ① Ø = Angle of internal friction of soil (degrees)
- ② N = Texas cone penetrometer value (blows per ft)
- (4) C(psi) = Cohesive shear strength of soil (psi)
- (5) C(psf) = Cohesive shear strength of soil (psf)





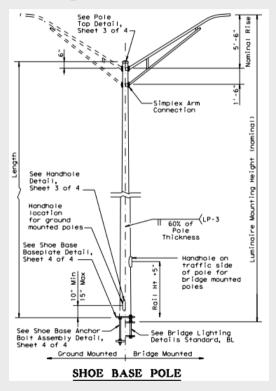






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Roadway Illumination Pole



| TABLE 2 | | | | |
|---|------------------------------------|----|----|--|
| RECOMMENDED FOUNDATION LENGTHS (See note 1) | | | | |
| MOUNT ING HE I GHT | TEXAS CONE PENETROMETER N Blows/f† | | | |
| HE TOHT | 10 | 15 | 40 | |
| <20 ft. | 6′ | 6' | 6′ | |
| >20 ft. to 30 ft. | 8, 6, 6, | | | |
| >30 ft. to 40 ft. | 8 | 8' | 6′ | |
| >40 ft. to 50 ft. | 10' | 8, | 6′ | |

| TABLE 3 | | | | | |
|---|--------------------|-------------------------|--|--|--|
| PAY QUANTITY OF RIPRAP PER FOUNDATION (Install only when shown on the plans) | | | | | |
| Foundation Diameter | RIPRAP DIAMETER | RIPRAP (CONC) (CL B) | | | |
| 30 in. | 78 in. | 0.35 CY | | | |

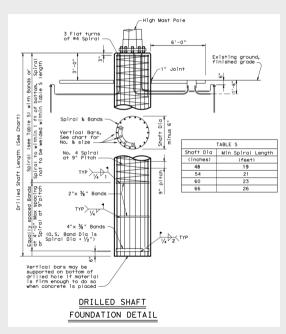
^{1. &}quot;Recommended Foundation Lengths" table is for information purposes only. Foundation lengths shall be as shown on the plans, or as directed by the Engineer. Foundations will be paid for under Item 416, "Drilled Shaft Foundations," unless otherwise shown on the plans.



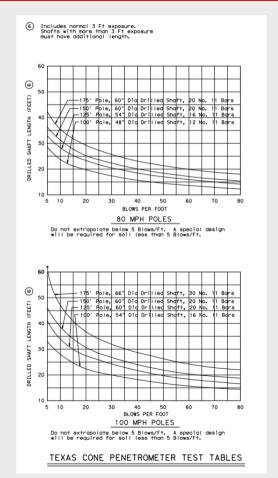
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High Mast Illumination Pole





Drilled shaft lengths as determined from the foundation design chart or other acceptable methods are to be as shown elsewhere on the plans.





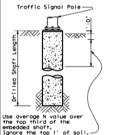
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Traffic Signal Pole

| | | | | | | | FOUND | MOITA | DESI | GN T | ABLE | | | | Г |
|---|------|---------|--------------|-------------------|-------|--------------------------------|---------------------|-----------------------|-------------|--------------------|----------------|----------------|---------------|--|-----|
| I | FDN | DRILLED | | FORCING TEEL | LENGT | D DRILLE | (5),(6) | | HOR BO | LT DES | IGN | FOUNDA DESI | TION IGN ② | | 1 (|
| | TYPE | SHAFT | VERT BARS | SPIRAL & PITCH | I 1 | ONE PENET blows/f 15 | TROMETER 1 40 | ANCHOR BOLT DIA | Fy (ksi) | BOLT CIR DIA | ANCHOR TYPE | MOMENT | SHEAR Kips | TYPICAL APPLICATION |] , |
| | 24-A | 24" | 4-#5 | #2 at 12" | 5.7 | 5.3 | 4.5 | ¾" | 36 | 12 ¾- | 1 | 10 | 1 | Pedestal pole, pedestal mounted controller. |] |
| ı | 30-A | 30" | 8-#9 | #3 at 6" | 11.3 | 10.3 | 8.0 | 1 1/2" | 55 | 17" | 2 | 87 | 3 | Most orm ossembly. (see Selection Table) |] (|
| | 36-A | 36" | 10-#9 | #3 at 6" | 13.2 | 12.0 | 9.4 | 1 ¾" | 55 | 19" | 2 | 131 | 5 | Most arm assembly, (see Selection Table) 30' strain pole with or without luminaire |] |
| | 36-B | | | #3 at 6" | 15.2 | 13.6 | 10.4 | 2" | 55 | 21" | 2 | 190 | 7 | Mast arm assembly, (see Selection Table) Strain pole taller than 30' & strain pole with mast arm |] (|
| ı | 42-A | 42" | 14-#9 | #3 ot 6" | 17.4 | 15.6 | 11.9 | 2 1/4" | 55 | 23" | 2 | 271 | 9 | Most orm assembly, (see Selection Table) | 1 |

| | FOUNDATION SELE ARM PLUS IL | CTION TABL SN SUPPORT | E FOR STANDA ASSEMBLIES | ARD MAST (ft) | |
|-------------------|--------------------------------|--------------------------|----------------------------|------------------|-----------|
| | | FDN 30-A | FDN 36-A | FDN 36-B | FDN 42-A |
| Z | MAX SINGLE ARM LENGTH | 32' | 48' | | |
| DESIGN SPEED | | 24' X 24' | | | |
| ISE | | 58, X 58, | | | |
| 1=0 | | 32' X 28' | 32' X 32' | | |
| 80 MPH | LENGTH COMBINATIONS | | 36, x 36, | | |
| l g ≆ | | | 40' X 36' | | |
| Ι- | | | 44' X 28' | 44' X 36' | |
| z | MAX SINGLE ARM LENGTH | | 36' | 441 | |
| SPEED SPEED | | | 24' X 24' | | |
| leg | | | 28' X 28' | | |
| Ξ× | MAXIMUM DOUBLE ARM | | 32' X 24' | 32' X 32' | |
| 100 MPH WIND S | LENGTH COMBINATIONS | | | 36' X 36' | |
| ls≆ | | | | 40' x24' | 40' X 36' |
| - | | | | | 44' x 36' |



| ı | NOTES: |
|---|--|
| | Anchor bolt design develops the foundation capacity given under Foundation Design Loads. |

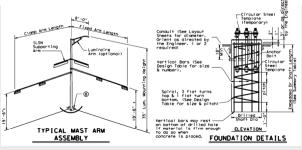
- ② Foundation Design Loads are the allowable moments and shears at the base of the structure.
- Toundations may be listed separately or grouped occording to similarity of location and type. Quantities are for the Contractor's information only.

 Field Penetrometer readings at a depth of approximately 3 to 5 feet may be used to adjust shart lengths.
- (5) If rock is encountered, the Drilled Shoft shall extend a minimum of two diameters into solid rock.
- (6) Decimal lengths in Design Table are to allow interpolation for other penetrometer values. Round to nearest foot for entry into Summary Table.

| | ANCHOR BOLT & TEMPLATE SIZES | | | | | | | | | | | | | | |
|--------------------|------------------------------|---------------|------------------|----------------|---------|--------|--|--|--|--|--|--|--|--|--|
| BOLT DIA IN. | ① BOLT LENGTH | TOP THREAD | BOTTOM THREAD | BOLT CIRCLE | Re | R) | | | | | | | | | |
| } /4" | 1'-6" | 3" | _ | 12 3/4" | 7 1/8" | 5 % " | | | | | | | | | |
| 1 1/2" | 3'-4" | 6" | 4" | 17" | 10" | 7" | | | | | | | | | |
| 1 ¾" | 3'-10" | 7" | 4 1/2" | 19" | 11 1/4" | 7 1/4" | | | | | | | | | |
| 2- | 4'-3" | 8* | 5" | 21" | 12 1/2" | 8 1/2" | | | | | | | | | |
| 2 1/4" | 4'-9" | 9" | 5 1/2" | 23- | 13 1/4" | 9 1/4" | | | | | | | | | |

Min dimensions given, longer bolts are acceptable.

| FO | UNDA | TION | ı Su | MMAR | Y TA | BLE | 3 | | | | | | |
|----------------------------|-----------------------------|------|------|--------|---------------|------|------|----------|--|--|--|--|--|
| LOCATION IDENTIFICATION | AVG. | FDN | NO. | (FEET) | | | | | | | | | |
| IDENTIFICATION | /ft. | TYPE | EA | 24-A | 30-A | 36-A | 36-B | 42-A | | | | | |
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| | | | | | | | | | | | | | |
| TOTAL DRILLED | TOTAL DRILLED SHAFT LENGTHS | | | | | | | | | | | | |
| | | pa t | | | | | | | | | | | |

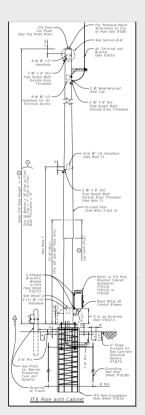




| Structure | Standard | Soil Strength Parameters | Design Loading Conditions |
|---|---------------------------|---------------------------------|-----------------------------------|
| Large Road Signs | SMD (8W2) - 08 | N _{TCP} , <i>C</i> , φ | Not Specified |
| Overhead Sign Bridge | OSB - FD OSB - FD - SC | N _{TCP} , <i>C</i> , φ | Uplift Loading and Moment |
| Monotube Sign Structure (Cantilever) | MC(7) - 22 | N _{TCP} | Axial, Moment, Torsion, and Shear |
| Monotube Sign Structure (Span) | MS(7) - 22 | N_{TCP} | Axial, Moment, Torsion, and Shear |
| Cantilever Overhead Sign Support | COSS - FD | N _{TCP} , <i>C</i> , φ | Moment and Torsion |
| Roadway Illumination Pole | RID(2) - 20 | N_{TCP} | Not Specified |
| High Mast Illumination Pole | HMIF(2) - 98 | N_{TCP} | Not Specified |
| Traffic Signal Pole | TS - FD - 12 | N_{TCP} | Moment and Shear |
| Intelligent Transportation Systems Pole | ITS(4) - 15 | N _{TCP} | Not Specified |



Intelligent Transportation Systems Pole



| | TABLE 2: ITS POLE - 110 MPH (W/ 2 SOLAR PANELS) ① | | | | | | | | | | | | | | | | | | | |
|--------------|---|--------------------------------|-----------------------------|-------------------------------|------------------------|----------------------|--------------------------------|------------------------------|-----------------------|----------------------|--------------|-----------------|--------------------------------|---------------------------------|----------------------------------|---------------------------|---------|-------------------------------------|----------|-------------------------------|
| | | PO | LE SHAFT | 10 | | BA | E (1) | | TOP ② | | | A | NCHOR BOLT | 3 | | FOUNDATION 3 | | | | |
| POLE TYPE | | BOTTOM OUTSIDE DIA. (IN) | TOP OUTSIDE DIA. (IN) | WALL THICK NESS (IN) | INSIDE DIA. (IN) | OUTSIDE DIA. (IN) | BOLT CIRCLE DIA. (IN) | BOLT HOLE DIA. (IN) | THICK NESS (IN) | OUTSIDE DIA. (IN) | DIA. (IN) | NO. OF BOLTS | LENGTH OF BOLT MIN. (IN) | TEMPLATE INSIDE DIA. (IN) | TEMPLATE OUTSIDE DIA. (IN) | TEMPLATE WIDTH (IN) | CONE PI | AFT DEPTI ENETROME 'FT.) (SEE | TER (N - | DRILLED SHAFT DIA. (IN) |
| | 'A' | 'B' | 'C' | 'D' | 'E' | 'F' | 'G' | 'H' | T | · Jr | 'K' | T. | 'M' | 'N' | '0' | 'P' | N = 10 | N = 15 | N = 40 | 'R' |
| | 20 | 10 | 8 | 1/2 | 10-1/16 | 21 | 16 | 1-1/4 | 1-1/2 | 9 | 1 | 4 | 29 | 14 | 18 | 2 | 14 | 12 | 10 | 36 |
| | 30 | 13 | 9 | 1/2 | 13-1/16 | 24 | 19 | 1-9/16 | 1-3/4 | 10 | 1-1/4 | 6 | 35 | 16-1/2 | 21-1/2 | 2-1/2 | 18 | 15 | 11 | 36 |
| ЕD | 40 | 15 | 9 | 1/2 | 15-1/16 | 25 | 21 | 1-9/16 | 1-3/4 | 10 | 1-1/4 | 6 | 35 | 18-1/2 | 23-1/2 | 2-1/2 | 20 | 17 | 12 | 42 |
| SIDED | 45 | 16 | 10 | 1/2 | 17-1/16 | 27 | 22 | 1-9/16 | 1-3/4 | 11 | 1-1/4 | 8 | 35 | 19-1/2 | 24-1/2 | 2-1/2 | 21 | 18 | 13 | 42 |
| 80 | 50 | 17 | 10 | 1/2 | 18-1/16 | 28 | 23 | 1-9/16 | 1-3/4 | 11 | 1-1/4 | 8 | 35 | 20-1/2 | 25-1/2 | 2-1/2 | 22 | 19 | 14 | 42 |
| | 55 ⑦ | 19 | 11 | 5/8 | 19-1/16 | 30 | 25 | 1-9/16 | 2 | 12 | 1-1/4 | 8 | 35 | 22-1/2 | 27-1/2 | 2-1/2 | 24 | 20 | 14 | 42 |
| | 60 ⑦ | 20 | 11 | 5/8 | 20-1/16 | 31 | 26 | 1-13/16 | 2 | 12 | 1-1/2 | 6 | 40 | 23 | 29 | 3 | 25 | 21 | 15 | 48 |

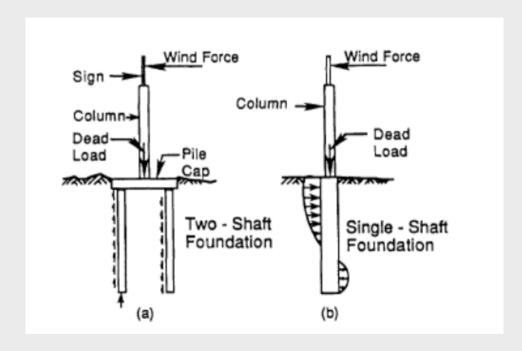
| | | | | | | | TAE | 3LE 3: | | | | PH (V | // 1 50L | AR PANE | L) ⑤ | | | | | |
|--------------|------------------------|--------------------------------|-----------------------------|-------------------------------|------------------------|----------------------|--------------------------------|------------------------------|-----------------------|----------------------|---------------|-----------------|--------------------------------|---------------------------------|----------------------------------|---------------------------|--------------|-------------------------------------|----------|-------------------------------|
| | | PO | LE SHAFT | 10 | BASE PLATE (1) | | | | | TOP ② PLATE | ANCHOR BOLT ③ | | | | | | FOUNDATION 3 | | | |
| POLE TYPE | POLE HEIGHT (FT) | BOTTOM OUTSIDE DIA. (IN) | TOP OUTSIDE DIA. (IN) | WALL THICK NESS (IN) | INSIDE DIA. (IN) | OUTSIDE DIA. (IN) | BOLT CIRCLE DIA. (IN) | BOLT HOLE DIA. (IN) | THICK NESS (IN) | OUTSIDE DIA. (IN) | DIA. (IN) | NO. OF BOLTS | LENGTH OF BOLT MIN. (IN) | TEMPLATE INSIDE DIA. (IN) | TEMPLATE OUTSIDE DIA. (IN) | TEMPLATE WIDTH (IN) | CONE PI | AFT DEPTH ENETROMET FT.) (SEE | 'ER (N - | DRILLED SHAFT DIA. (IN) |
| | 'A' | 'B' | 'C' | 'D' | 'E' | 'F' | 'G' | 'H' | T | 'J' | 'K' | 'L' | 'M' | 'N' | '0' | 'P' | N = 10 | N = 15 | N = 40 | 'R' |
| | 20 | 10 | 8 | 1/2 | 10-1/16 | 21 | 16 | 1-9/16 | 1-3/4 | 9 | 1-1/4 | 4 | 35 | 13-1/2 | 18-1/2 | 2-1/2 | 16 | 14 | 10 | 36 |
| | 30 | 13 | 9 | 1/2 | 15-1/16 | 24 | 19 | 1-9/16 | 1-3/4 | 10 | 1-1/4 | 6 | 35 | 16-1/2 | 21-1/2 | 2-1/2 | 18 | 16 | 11 | 36 |
| ED | 40 | 15 | 9 | 1/2 | 15-1/16 | 26 | 21 | 1-9/16 | 1-3/4 | 10 | 1-1/4 | 6 | 35 | 18-1/2 | 23-1/2 | 2-1/2 | 21 | 18 | 13 | 42 |
| SIDED | 45 | 16 | 10 | 1/2 | 16-1/16 | 27 | 22 | 1-9/16 | 1-3/4 | 11 | 1-1/4 | 8 | 35 | 19-1/2 | 24-1/2 | 2-1/2 | 23 | 19 | 14 | 42 |
| 00 | 50 | 17 | 10 | 1/2 | 17-1/16 | 28 | 23 | 1-9/16 | 2 | 11 | 1-1/2 | 8 | 40 | 20 | 26 | 3 | 24 | 20 | 14 | 42 |
| | 55 ⑦ | 19 | 11 | 5/8 | 19-1/16 | 30 | 25 | 1-13/16 | 2 | 12 | 1-1/2 | 8 | 40 | 22 | 28 | 3 | 27 | 22 | 15 | 42 |
| 1 | 60 (7) | 20 | 11 | 5/8 | 20-1/16 | 31 | 26 | 1-13/16 | 2 | 12 | 1-1/2 | 8 | 40 | 23 | 29 | 3 | 28 | 23 | 16 | 48 |

5. Recommended embedment lengths are for information purposes only. Foundation embedment depth is based off Texas Cone Penetrometer Value N = 10 blows/ft. for soft soils and up to 40 blows/ft. for hard soils. Foundation lengths shall be as shown on the plans, or as directed by the Engineer. Foundations will be paid for under Item 416, "Drilled Shaft Foundations" unless otherwise shown on the plans.



Design of Laterally Loaded Structures

- Design philosophy different than bridges
 - Approach differs from axial capacity design
 - Brom's method
 - P-Y analysis
 - Transient load controlled
 - Often designed assuming loads are static
 - Often wider boring spacing

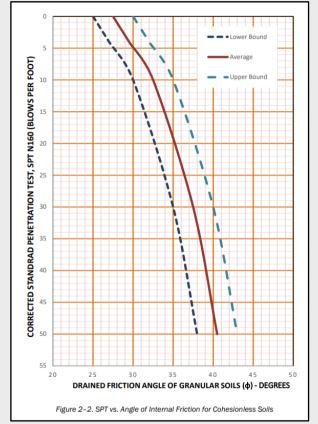


LRFD Update

- All standards currently use N_{TCP} , C, or Φ to determine foundation length
- Long-term, standards will be updated to LRFD
 - TCP → SPT
- Short-term, use <u>Geotechnical Manual LRFD (April 2024)</u>
 - Figure 2-2: SPT vs Angle of Internal Friction for Cohesionless Soils
 - Recommended to use "Lower Bound" of Φ
 - Appendix 2: N_{TCP} vs N_{SPT} correlation

In Clay:
$$N_{TCP} = 1.5 \times N_{SPT}$$

In Sand:
$$N_{TCP} = 2.0 \times N_{SPT}$$





Soil Property Selection Best Practices

- Boring proximity
 - New boring with SPT and laboratory testing
 - Existing TCP boring
- Soil type
- Soil uniformity with depth
- Future improvements/changes of conditions
- Shrinking/swelling soils





Examples

- Overhead Sign Bridges (OSB)
 - Axial (Uplift) Load
- Cantilever Overhead Sign Structure (COSS)
 - Lateral and Torsional Load







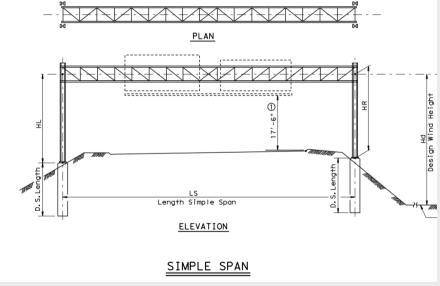
Overhead Sign Bridges (OSB)



Design Loading Conditions: Uplift Loading and Moment

Foundation Diameter: 24" - 54"

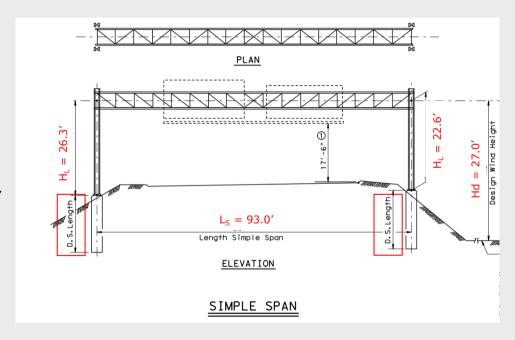
Foundation Embedment: 4' - 55'





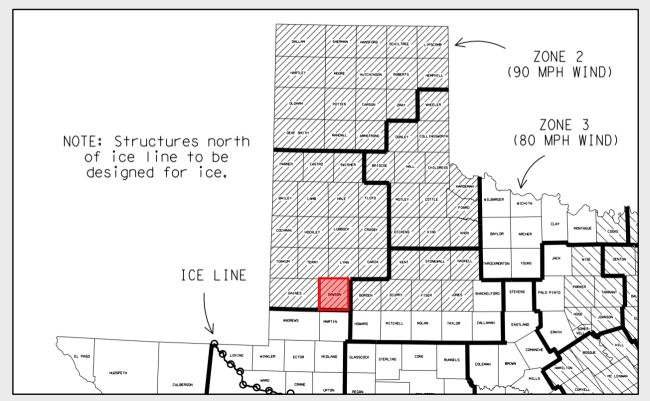
Overhead Sign Bridges (OSB)

- Given:
 - Span, $L_S = 93.0'$
 - Left Tower Height, $H_1 = 26.3'$
 - Right Tower Height, $H_R = 22.6'$
 - Design Wind Height, $H_d = 27.0'$
 - Avg. <u>SPT</u> Penetrometer Value,N = 20
 - Dawson County



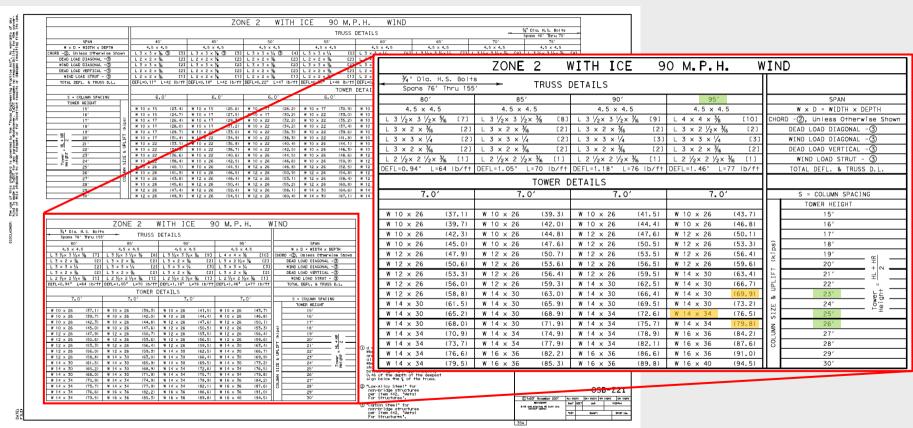


Overhead Sign Bridges (OSB)

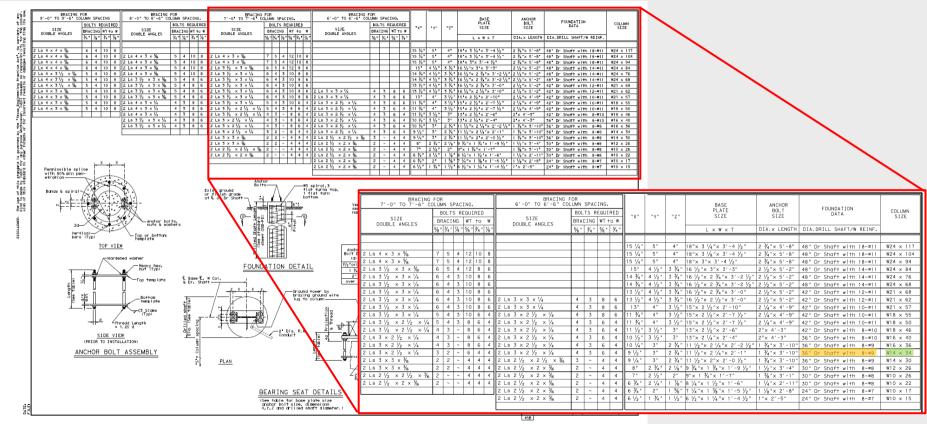


- Zone 2
- 90 mph
- Design Height, Hd = 27.0'

Standard: OSB-Z2I







Appendix 2

Ancillary Structure Foundations

When using roadway and traffic standards developed for foundations from TCP information (COSS, High Mast Illumination Poles), use the following correlations (from *Touma and Reese, 1972*) from SPT values acquired in the drilled boring logs:

In Clay:
$$N_{TCP} = 1.5 * N_{SPT}$$

Where, N_{TCP} = equivalent TCP blow counts when using STP information

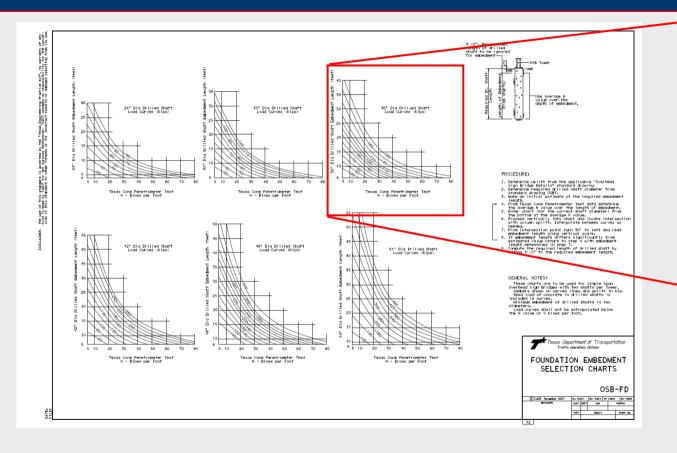
N_{SPT} = uncorrected blow counts from STP in-situ testing

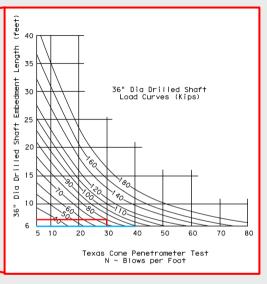
These correlations apply to the standard foundation embedment selection charts regarding TCP information currently refenced in the standards.

$$N_{SPT} = 20 bl$$

In Clay:
$$N_{TCP} = 30 \text{ bl}$$

In Sand: $N_{TCP} = 40 \text{ bl}$





$$Uplift_{L} = 80 \text{ k}$$

$$Uplift_{R} = 70 \text{ k}$$

$$L_L = 7' + 3' = 10'$$

 $L_R = 6' + 3' = 9'$

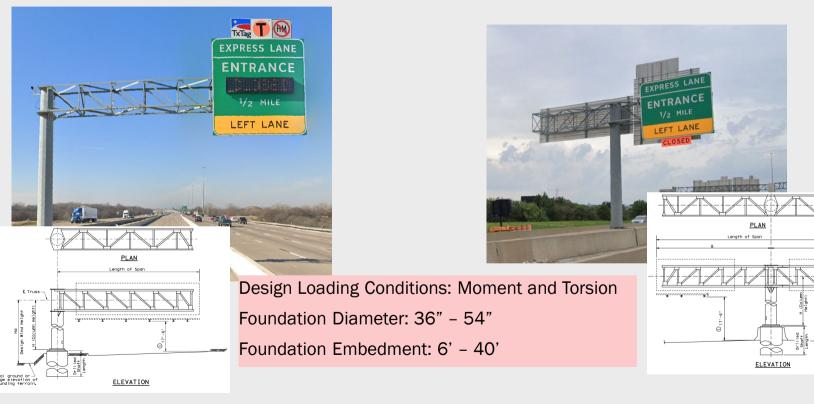
Sand (40 bl)

$$L_L = 6' + 3' = 9'$$

 $L_R = 6' + 3' = 9'$



Cantilever Overhead Sign Structures (COSS)

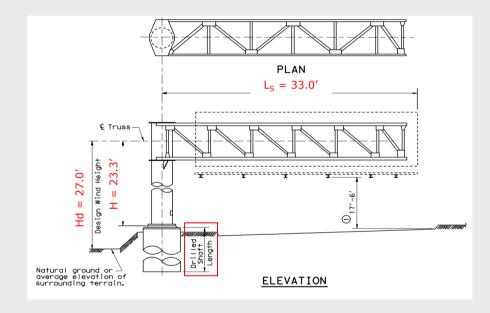


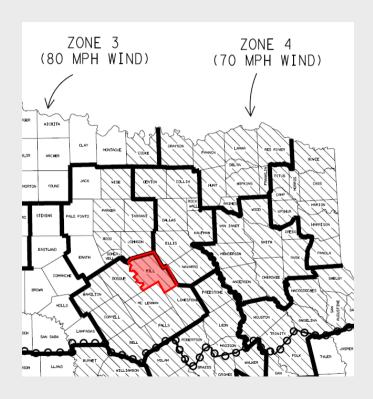


Cantilever Overhead Sign Structures (COSS)

• Given:

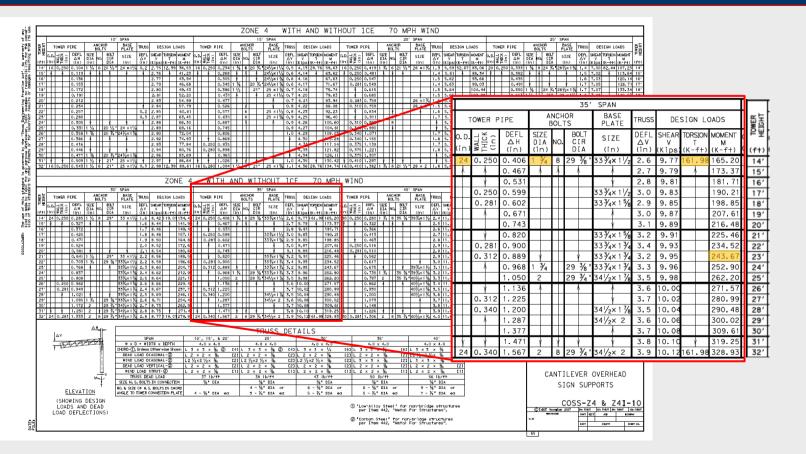
- Cantilever Span = 33.0'
- Column Height, H = 23.3'
- Design Wind Height, $H_d = 27.0'$
- Avg. <u>SPT</u> Penetrometer Value,N = 15
- Hill County



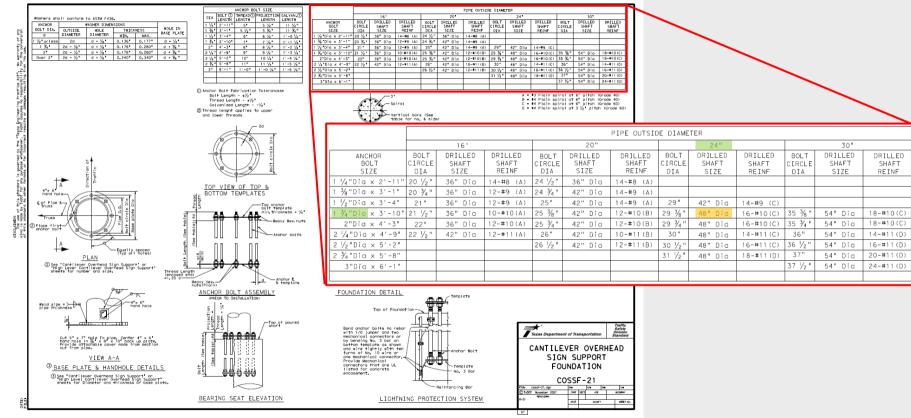


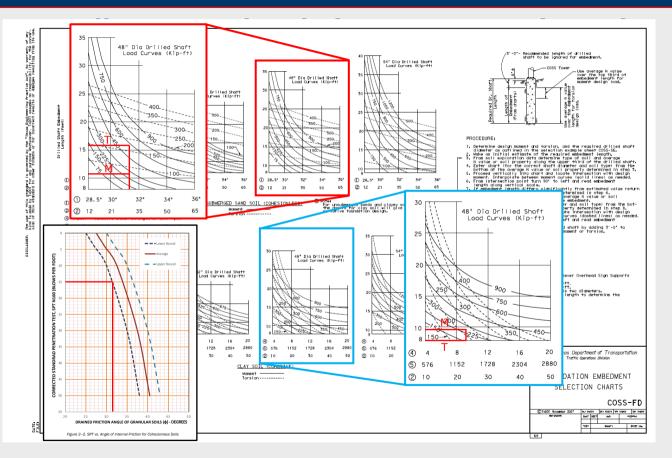
- Zone 4
- 70 mph
- Design Height, Hd = 27.0′

Standard: COSS-Z4 & Z4I-10









Torsion = 162 k-ft Moment = 244 k-ft N_{SPT} = 15 bl

Sand:

Method 1:

$$N_{TCP} = 2 \times N_{SPT} = 30 \text{ bl}$$

 $L = 16' + 3' = 19'$

Method 2:

$$N_{SPT} = 15 \text{ bl}$$

$$\downarrow$$
Figure 2-2 (Lower Bound)
$$\downarrow$$

$$\phi = 31^{\circ}$$

$$L = 16' + 3' = 19'$$

Clay:

$$N_{TCP} = 1.5 \times N_{SPT} = 22.5 \text{ bl}$$

 $L = 10' + 3' = 13'$



Future Plans

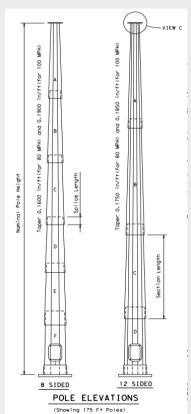
- Research FY25
 - Refine design approach compliant with current AASHTO requirements
 - Simplified soil parameters for LRFD based design approach
- Standard Updates
 - Design updates
 - Future standards to add N_{SPT} and/or C and Φ



Conclusions

- Every standard is different
 - Read the general notes on usage
- Geotechnical parameter selection requires discernment
 - Use all the information you have
- Keep an eye out for additional updates
- Feel free to reach out if you have questions







Questions?

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