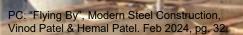
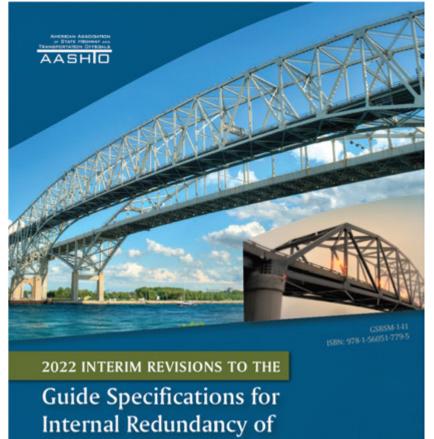
NLCOR®

IRM CASE STUDY BUILT-UP STEEL BENT





Mechanically-Fastened Built-Up Steel Members

2018 First Edition

	- Section B-B	Date:	-
sumptions:			
		nored. 3. Maximum of two holes in each leg of	the angles.
Outermost cover plate in tension is the faile	d component.		
neral Requirements (GS 1.4 and 1.5)		Clear Al	Inputs Clear Gray Cells
Does the member exhibit severe concision Does the member meet all other provision Does the member meet all provisions of S	ut of Section 14?	Y	No No es Tur es Tur
ember Cross Section Inputs			
inner at			
Steel yield strength, F.	58.8 -	Fastener type	Bolt teltartinet
Steel tensile strength, F.	78.0	Are holes punched full size?	No faratta
Is the girder composite with the deck?	No forethe	Fastener diameter, d'aut	1.0000 -
Include haunch in section properties?	No Turantia	Fastener hole diameter, d'an	11250 -
Is this for a negative moment region?	No Turatta	Is compression flange welded to web?	Yes Turotta
eference Sketches:			
	PL2	RJ RJ	- Anana
		1 1	E 1
		~	1 3
won-	Web PL		1
WOR	Web PL	-	14
won	Web PL		145
	R.4		
	R4-		<u> </u>
oncrete Deck Properties: Elective Sub Vide, A,	R4	- P. 3	
nected Deck Properties: Directive Stab Vidb, A, Deck Stab Trickness, 7,		Haush Vills, has	
n,1 n,2	R1 R2	- P. 3	
oncrete Deck Properties: Elective Stab Vidh, <i>J.</i> , Deck Stab Teckness, <i>I</i> , Concrete Modules of Elasticity, <i>E</i> ,	R.1 R.2	Hauch Vidh, bt Hauch Vidh, bt Hauch Thickness, tt Moddar Ratio of the concrete, <i>o</i>	
ncrete Deck Properties: Dirctive Stab Vidh, A. Dirctive Stab Vidh, A. Concrete Modules of Elarticity, F. Concrete Modules of Elarticity, F.	R.1 R.2	Haush Vills, has	
oncrete Deck Properties: Directive Stab Vidh, A. Deck Stab Widh, A. Deck Stab Dirkness, C, Concrete Modulas of Elasticity, F, ongression Flange Plate Dimension to of Comp. Flange Cover Plates, N.,	R.4 R.1 R.2 · · · ·	Pr. 3 Pr. 1 Haunch Vilöh, bast Haunch Tinkhness, tast Moddar Ratio of the concrete, <i>o</i> Compression Flange Angle Proper	
Deck Stab Vidh, <i>A</i> , Deck Stab Vidh, <i>A</i> , Deck Stab Thickness, <i>t</i> , Concrete Modulas of Elasticity, <i>E</i> , omgression Flange Plate Dimensis o. of Comp. Flange Cover Plates, <i>N</i> _{exc} line(f)	R.4 R.1 R.2 · · · ·	Haunch Vidb, hann Haunch Vidb, hann Haunch Thickness, tank Modular Ratio of the concrete, J Compression Flange Angle Proper Select the size of the angles	
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oncrete Deck Properties: Directive Slab Vidh, A. Concrete Modules of Elasticity, F. concrete Modules of Elasticity, F. ongression Flange Plate Dimensio to: of Comp. Flange Cover Plates, N ter(i) 2 2 3 A	R.1 R.2	P. 3 P. 3 P. 1 Haunch Vidh, hk Haunch Thikkness, kk Moddar Ratio of the concrete, <i>n</i> Compression Flange Angle Proper Select the size of the angles Select the size of the angles Distribution of the long lang Gross area of a single angle, <i>A</i> , Thickness of the angle, <i>A</i> , Moment of lanets about holis, asis, <i>A</i> ,	Tark.
Oncrete Deck Properties: Directive Slab Vidh, A. Deck Slab Vidh, A. Deck Slab Transe Plate Dimension (on Comp. Flange Plate Dimension (on Comp. Flange Cover Plates, N. Intel() S.0.00 The Plate Is absorbed astermant (P. for	R.4 R.1 R.2	Haunch Vidb, h	Tark.
Concrete Deck Properties: Elective Stab Vidb, A. Deck Stab Vidb, A. Deck Stab Teickness, /, Concrete Modular of Elections, F., Compression Flange Plate Dimension to of Comp. Fingh Cover Plates, N., Nare() 0.0.00 A., 2.500	R.4 R.1 R.2	P. 3 P. 3 P. 1 Haunch Vidh, hk Haunch Thikkness, kk Moddar Ratio of the concrete, <i>n</i> Compression Flange Angle Proper Select the size of the angles Select the size of the angles Distribution of the long lang Gross area of a single angle, <i>A</i> , Thickness of the angle, <i>A</i> , Moment of lanets about holis, asis, <i>A</i> ,	Tark.
Concrete Deck Properties: Elective Stab Vidb, <i>A</i> , Deck Stab Thickness, <i>I</i> , Concrete Modulus of Electricity, <i>E</i> , Compression Flange Plate Dimension to of Comp. Flange Over Plates, <i>N</i>	R.1 R.2	Haunch Vidb, huma Haunch Vidb, huma Haunch Thickness, fumi Modular Ratio of the concrete, Jo Compression Flange Angle Proper Select the size of the angles Select the orientation of the long leg Gross area of a single angle, A Moment of inertia about hold, sels, fumi Outer Hold, Leg to N.A., 5.	7
Concrete Deck Properties: Elective Slab Vidh, A. Concrete Modulus of Elasticity, F. Concrete Modulus of Elasticity, F. Congression Flange Plate Dimensio to of Comp. Finge Cover Plates, N. Mart(0) Face Education Structure Structure Compression Flange Angle Fastence Holes in Vert Angle Leg, N. Amardia	R 1 R2	Haunch Vildh, bt Haunch Vildh, bt Haunch Thickness, tt Moddar Ratio of the concrete, <i>a</i> Compression Flange Angle Proper Select the size of the angles Subscr the orientation of the long leg Gross area of a single angle, A Thickness of the angle, 7 Moment of inertia about holit, anit, 7 Outer Hotiz, Leg to N.A., 5 Holes in Hotz Angle Leg, Manufac	14 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
Pr. 1 Pr. 2 Pr. 1 Pr. 1 Pr. 2 Pr. 1	R.4 R	Haunch Vidh, bs Haunch Vidh, bs Haunch Thickness, ts Modular Ratio of the concrete, <i>A</i> Compression Flange Angle Proper Select the size of the angles Select the coincration of the long leg Gross area of a single angle, <i>A</i> s Thickness of the angle, <i>A</i> Thickness of the angle, <i>A</i> Thickness of the angle, <i>A</i> Moment of Isertia about hoitz aits, <i>A</i> Outer Hotic, Leg to N.As. Holes in Hotz Angle Leg, <i>N</i> Annuh Distance to Hoite L of sites	14 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1

METHODOLOGY

4

- 1. Screening Criteria
- 2. Check fatigue life in *unfaulted* state
- 3. Check strength in *faulted* state
- 4. Check fatigue life in *faulted* state
- 5. Calculate Special Inspection Interval

EXAMPLE DESIGN – STRADDLE BENT

- A709-50W steel
- Drilled 1-1/8" Φ bolt holes
- $F_y = 50$ ksi, $F_u = 70$ ksi
- $(ADTT)_{SL} = 2,000$

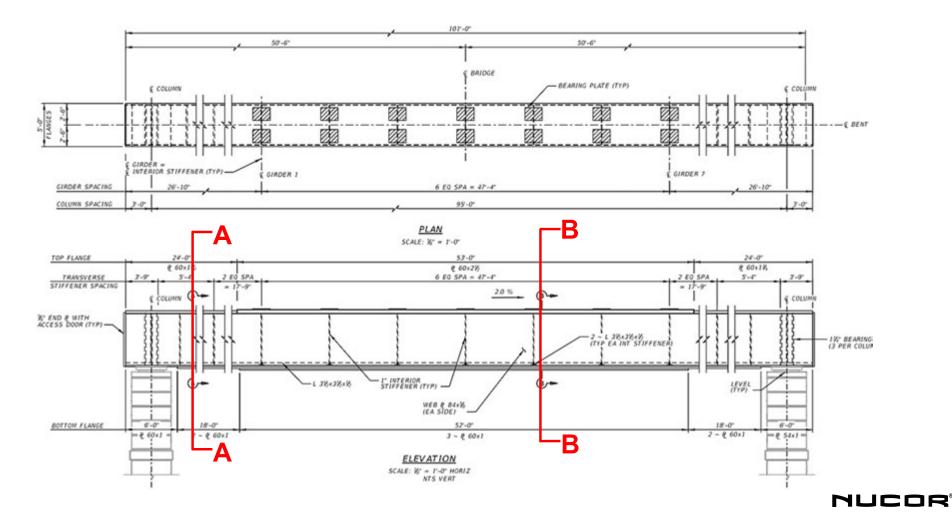
5

• g = 1% (annual ADTT_{SL} growth rate)

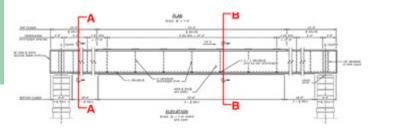


PC: Ronnie Medlock, High Steel

SPAN LAYOUT, (101 FT. LONG, 95' OC)



6



7

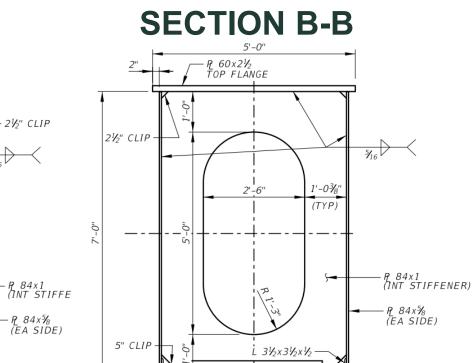
SECTION A-A

₽ 60x1¾ TOP FLANGE

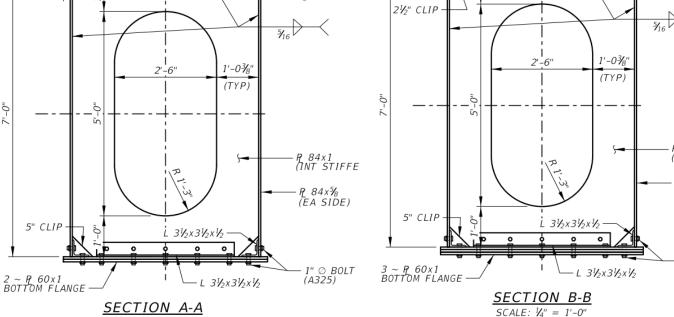
2"

7'-0"

5'-0"



- 1" ⊘ BOLT (A325)



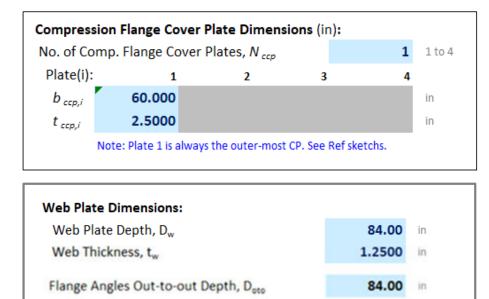
 $\frac{SECTION A-A}{SCALE: V_4'' = 1'-0''}$

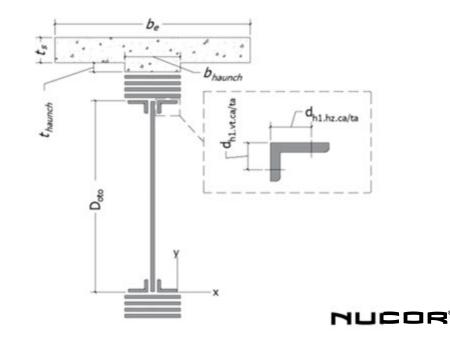
SECTION PROPERTIES (B-B)

General:

50.0	ksi
70.0	ksi
No	Yes or No
No	Yes or No
No	Yes or No
	70.0 No No

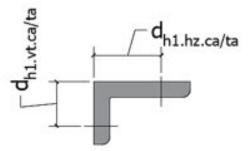
Fastener type	Bolt	Bolt or Rivet
Are holes punched full size?	No	Yes or No
Fastener diameter, d fast	1.0000	in
Fastener hole diameter, d hole	1.1250	in
Is compression flange welded to web?	Yes	Yes or No





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SECTION PROPERTIES CONT. (B-B)



Tension Flange Cover Plate Dimensions (in):								
No. of Te	nsion Flange (Cover Plates,	N tcp	3	1 to 4			
Plate(i):	1	2	3	4				
b tcp,i	60.000	60.000	60.000		in			
t _{tcp,i}	1.0000	1.0000	1.0000		in			
	Note: Plate 1 is always the outer-most CP, see Ref Sketches							

Tension Flange Angle Fastener Holes:



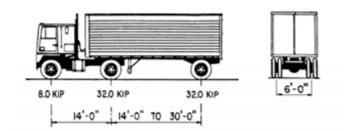
Tension Flange Angle Properties:

Select the size of the angles	L3-1/	2X3-1/2X1/2	
Select the orientation of the long	leg	Vert	V or H
Gross area of a single angle, A ta.g		3.25	in ²
Thickness of the angle, t ta		0.5	in
Moment of Inertia about horiz. a	Moment of Inertia about horiz. axis, I xx.ta		in ⁴
Outer Horiz. Leg to N.A., y _{ta}		1.05	in
Holes in Horz Angle Leg, N holes.hz.to	2	1	holes
Distance to Hole 1, $d_{h1.h2.ta}$		2.00	in
Distance to Hole 2, $d_{h2.hz.ta}$			in

IRM EVALUATION LOADS

Redundancy II Load Case:

- HL-93 design trucks (w/12 config.)
- IM = 15% (not on lane)
- Multiple presence factors
- $\gamma_{DC} = 1.05, \ \gamma_{DW} = 1.05, \ \gamma_{LL} = 1.30, \ \gamma_{FATI} = 1.75, \ \gamma_{FATII} = 0.80$



	Unfactored					
Section	M _{DC1} (kip-ft)	M _{LL+IM} (kip-ft)	M _{FAT+IM} (kip-ft)			
A-A	19,248	5,239	2,518			
B-B	31,500	8,574	4,122			

SECTION PROPERTIES CONT. (B-B)

Unfaulted Member Section Properties:

Assumed no failed components

Gross Section Properties: Unfaulted (composite if applicable)

Com	Composite		N	oncomposite	2
YG,COMP	-	in	Yc,g,NC	38.8	in
C _{G,COMP}	-	in	C _{G,NC}	41.8	in
A _{G,COMP}		in ²	AG,NC	441.5	in ²
IxG,COMP	-	in ⁴	I _{xG,NC}	705,928.0	in ⁴
S _{XG,COMP}	-	in ³	S _{xG,NC}	16,895.9	in ³

Faulted Member Section Properties:

Assumed failed component is Tension Cover PL 1 (outer-most cover plate)

Gross Section Properties: Faulted (composite if applicable)

Composite		N	oncomposite		
YAFG,COMP	-	in	VAFG,NC	45.3	in
CAFG,COMP		in	CAFG,NC	47.3	in
AAFG,COMP	-	in ²	AAFG,NC	381.5	in²
IX,AFG,COMP		in ⁴	IX,AFG,NC	587,594.8	in ⁴
S _{x-AFG,COMP}		in1	S _{x-AFG,NC}	12,429.7	in ³

Net Section Properties: Unfaulted (composite if applicable) Composite

- in

- in

in²

- in*

- in³

YN.COMP

CN.COMP

AN,COMP

N.COMP

S_{XN,COMP}

Com	posite	Noncomposite		
YAFN, COMP	- in	YAFN, NC	46.3	in
CAFN,COMP	- in	CAFN,NC	48.3	in
AAFN, COMP	- in ²	AAFN,NC	373.3	in²
IX, AFN, COMP	- in ⁴	IX,AFN,NC	570,201.9	in^4
S _{X-AFN,COMP}	- in ¹	Sx-AFN,NC	11,815.1	in ³

Net Section Properties: Faulted (composite if applicable)

Noncomposite

39.7 in

42.7 in

431.1 in²

689,092.8 In*

16,127.3 in³

YNNC

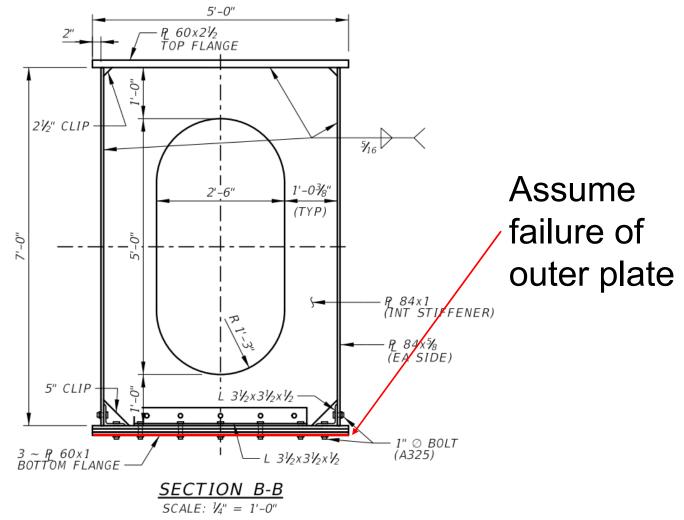
CN.NC

AN,NC

IXN.NC

SXN.NC





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UNFAULTED FATIGUE & FAULTED STRENGTH (B-B)

Total unfaulted state remaining fatigue life, Y REM	Infinite Year	5	MBE 7.2.5.1		Unfaulted	fatigue life?	OK	
Case I or II for unfaulted state?	l or II		G\$ 2.5					
Redundancy II Factored Moment:								
$M_{u} = \gamma_{DC}(M_{DC1} + M_{DC2}) + \gamma_{DW}M_{DW} + \gamma_{LL}M_{LL+IM} =$	44,221	k-ft	GS Eq. 1.7.1-1					
Factored Resistance:								
$f_{u,n} = \varphi_u F_u =$	56.00	ksi	GS Eq. 2.3-2					
$f_{u,g} = \varphi_{\gamma} F_{\gamma} =$	47.50	ksi	GS Eq. 2.3-1					
Factored Stress in the Faulted State: (<i>Note:</i> $\beta_{AF} = 1.0$)								
Noncomposite Section:								
$f_{AFN} = \beta_{AF} (M_u / S_{x - AFN}) =$	44.91	ksi	GS Eq. 2.1.2-1					
$f_{AFG} = \theta_{AF} (M_u / S_{x-AFG}) =$	42.69	ksi	GS Eq. 2.1.2-2					
			Strength Criteria Check:				Performance Ratio (D/C)	
			$f_{AFN} \leq f_{u,n}$?	ОК	OK or NG	GS Eq. 2.3-2	0.80	
			fAFG ≤ fug?	OK	OK or NG	GS Eq. 2.3-1	0.90	

METHODOLOGY CHECK-IN:

- 1. Screening Criteria PASS
- 2. Check fatigue life in unfaulted state INFINITE
- 3. Check strength in faulted state PASS
- 4. Check fatigue life in faulted state ?
- 5. Calculate Special Inspection Interval ?

FATIGUE LIFE IN FAULTED STATE

Effective Fatigue Stress Range & Max Fatigue Stress Range in Faulted State: Noncomposite Section: $f_{AFN} = \theta_{AF} (M_{FAT+IM} / S_{x-AFN}) = 4.8$ ksi

$(\Delta f)_{max} = (\gamma_{FATI}/\gamma_{FATII})(\Delta f)_{eff} = (1.75/0.8)(4.8ksi) = 10.5 ksi$ Use Cat. C (CATH = 10 ksi), therefore **FINITE** life

Total faulted state remaining fatigue life, Y REM	43.1	Years	MBE 7.2.5.1	Faulted fatigue life?	OK
Case I or II for faulted state?	I(b)	l(a), l(b), ll	GS 2.5		
Total Remaining Fatigue Life:					
No. of accumulated years in unfaulted state, N u	0.0	Years	GS 2.5.3		
Total fatigue life only in unfaulted state, Y _u	60	Years	GS 2.5.3		
Total remaining fatigue life only in faulted state, Y _f	43.1	Years	GS 2.5.3		
Total remaining fatigue life, $N_f = Y_f (1 - N_u / Y_u)$	43.1	Years	GS Eq. 2.5.3-1		
Total remaining fatigue life, $N_f = r_f (1 - N_u / r_u)$	43.1	Tears	G5 Eq. 2.5.5-1		

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GS Eq. 2.1.2-1

SECTION B-B RESULT SUMMARY

- <u>Passed</u> screening criteria
- Possessed positive fatigue life unfaulted state
- <u>Passed</u> strength checks faulted state
- Possessed <u>positive</u> fatigue life faulted state

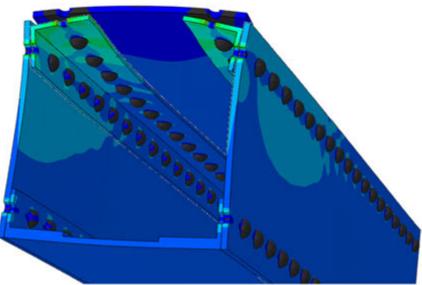
Qualifies as IRM

• Next – calculate Special Inspection Interval

SUMMARY & INSPECTION INTERVAL (B-B)

Summary of Results

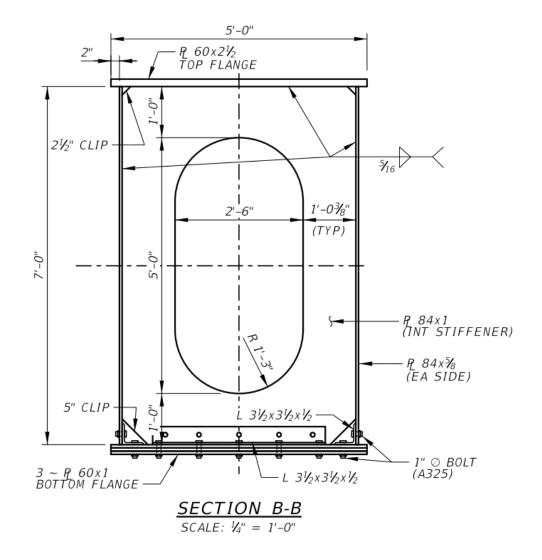
Strength check =	OK	OK or NG	GS 2.1 & 2.3
Fatigue case =	I(b)	l(a), l(b), ll	GS 2.5
Stress range in unfaulted state, $\Delta f_{UFS} =$	2.42	ksi	
Controlling stress range in faulted state, Δf_{FS} =	4.72	ksi	
Controlling faulted state remaining fatigue life, Y _{RDM}	45.1	Years	
Total remaining fatigue life, N _f	45.1	Years	GS Eq. 2.5.3-1
			100101
Maximum Interval for Special Inspections =	10.0	Years	GS 3



SUMMARY OF EVALUATION RESULTS

Section ID	Faulted Condition Strength Checks	IRM Special Inspection Interval
A-A	OK	
B-B	OK	

IS THERE A MORE EFFICIENT/ECONOMICAL DESIGN?



	Assumed Fractured					
Trial	Plate 1 Thickness (in.)	Plate 2 Thickness (in.)	Plate 3 Thickness (in.)	Meets Strength Req.?	Remaining Fatigue Life in Faulted Condition	Special Inspection Interval
1	1.0	1.0	1.0	Yes	45.1 yrs	10 yrs

	Assumed Fractured					
Trial	Plate 1 Thickness (in.)	Plate 2 Thickness (in.)	Plate 3 Thickness (in.)	Meets Strength Req.?	Remaining Fatigue Life in Faulted Condition	Special Inspection Interval
1	1.0	1.0	1.0	Yes	45.1 yrs	10 yrs
2	0.625	0.875	1.5	Yes	Infinite Life	10 yrs

	Assumed Fractured					
Trial	Plate 1 Thickness (in.)	Plate 2 Thickness (in.)	Plate 3 Thickness (in.)	Meets Strength Req.?	Remaining Fatigue Life in Faulted Condition	Special Inspection Interval
1	1.0	1.0	1.0	Yes	45.1 yrs	10 yrs
2	0.625	0.875	1.5	Yes	Infinite Life	10 yrs
3	1.5	1.5	-	No	-	-

Misses gross section yield limit by 7%

	Assumed Fractured					
Trial	Plate 1 Thickness (in.)	Plate 2 Thickness (in.)	Plate 3 Thickness (in.)	Meets Strength Req.?	Remaining Fatigue Life in Faulted Condition	Special Inspection Interval
1	1.0	1.0	1.0	Yes	45.1 yrs	10 yrs
2	0.625	0.875	1.5	Yes	Infinite Life	10 yrs
3	1.5	1.5	-	No	-	-
4	1.0	2.0	-	Yes	Infinite Life	10 yrs

	Assumed Fractured					
Trial	Plate 1 Thickness (in.)	Plate 2 Thickness (in.)	Plate 3 Thickness (in.)	Meets Strength Req.?	Remaining Fatigue Life in Faulted Condition	Special Inspection Interval
1	1.0	1.0	1.0	Yes	45.1 yrs	10 yrs
2	0.625	0.875	1.5	Yes	Infinite Life	10 yrs
3	1.5	1.5	-	No	-	-
4	1.0	2.0		Yes	Infinite Life	10 yrs

	Assumed Fractured					
Trial	Plate 1 Thickness (in.)	Plate 2 Thickness (in.)	Plate 3 Thickness (in.)	Meets Strength Req.?	Remaining Fatigue Life in Faulted Condition	Special Inspection Interval
1	1.0	1.0	1.0	Yes	45.1 yrs	10 yrs
2	0.625	0.875	1.5	Yes	Infinite Life	10 yrs
3	1.5	1.5	-	No	-	-
4	1.0	2.0		Yes	Infinite Life	10 yrs
5	1.25	1.75	-	Yes	39.7 yrs	10 yrs



IRM INSPECTION ADVANTAGE (RISK-BASED):

	Member Type	Reduced Intervals	Interval	Max Extended Interval (Method 2**)	Inspection Type
	LPRM	12 months (risk based)	48 months	72 months	Unchanged: routine
	NSTM	12 months (risk-based)	48 months	48 months	Unchanged: hands- on
	SRM	12 months (risk based)	48 months	72 months	Routine
*	IRM	12 months (risk based)	48 months		IRM Special Insp. (Routine inspection)

*Method 1: Interval determined by a "simplified" assessment of risk (criteria are defined in NBIS) **Method 2: Interval determined by a "more rigorous" assessment of risk using a Risk Assessment Panel (RAP) and documented as a formal policy.

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	Assumed Fractured					
Trial	Plate 1 Thickness (in.)	Plate 2 Thickness (in.)	Plate 3 Thickness (in.)	Meets Strength Req.?	Remaining Fatigue Life in Faulted Condition	Special Inspection Interval
1	1.0	1.0	1.0	Yes	45.1 yrs	10 yrs
2	0.625	0.875	1.5	Yes	Infinite Life	10 yrs
3	1.5	1.5	-	No	-	-
	1.0	2.8		Yes	Infinite Life	10 yrs
5	1.25	1.75	-	Yes	39.7 yrs	10 yrs
I-Beam	34 x 1.5	34 x 2	34 x 2	Yes	Infinite Life	10 yrs

IRM CONSIDERATIONS: COST & WEIGHT

COST APPROXIMATIONS: (Listed \$ to \$\$\$)

- 1. Fillet-welded box (fillets on outside of box only)
- 2. Built-up bolted I-girder (+1-2%)
- 3. Built-up bolted box girder (+15-18%)
- 4. CJP-welded box girder (+17-20%) -

Don't use CJPs!

WEIGHT APPROXIMATIONS: (Listed # to ###)

- 1. Built-up bolted I-girder
- 2. Welded box girders (10-14% heavier)
- 3. Built-up bolted box girder (16-20% heavier)

These are "front" costs only...consider life cycle costs (in-service inspection!) Consider POD of inspections...

Consider safety related to arms-length inspection requirements...

SAFETY

Bridge inspection is directly motivated by pursuit for safety

✤ 2015 Purdue study for INDOT found:

- Congested crash rate increased 24x on Indiana interstates with queue ≥ 5 min
- And safety of the inspection crews?
- Intend to find damage *before* it's an issue:
 - What about Probability of Detection (POD)?
 - Are we able to find what we think we can find?



FINAL THOUGHTS & WRAP-UP

REDUNDANCY & INSPECTION:

- Goes without saying that IRMs are more redundant than welded boxes
- Faulted state strength capacity calculations are very conservative (on-going research)
- Special inspection intervals are very conservative
 - 95% confidence interval, SF = 2, fracture propagation unlikely
- IRMs are robust against over height vehicle impacts
- POD of severed plate is very high changes the inspection reliability without compromising safety



THANK YOU

NUCOR CORPORATION



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