

Creating Useful Scour Documentation Yusuf Yurttas, P.E.



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What is scour

FHWA "Scour is the result of the erosive action of flowing water, excavating and carrying away material from the bed and banks of streams and from around the piers and abutments of bridges."







More things to consider

- Different materials scour at different rates, however ultimate scour depth can be similar across soil types.
- Maximum scour often occurs during peak floods and may take several floods.
- Scour holes can refill, hiding damage.





Scour at Bridges

- Erosion of streambed or bank material due to flowing water
 - Contraction scour
 - Constricting the channel at a bridge opening
 - Pier Scour
 - Obstructions to flow in the channel





The Leading Cause of Bridge Failures

Scour is the primary cause of bridge collapses during floods.

- 1987 floods: 17 bridges destroyed in NY and New England.
- 1985 floods: 73 bridges failed across PA, VA, and WV.
- 1993 Mississippi flood: 23 bridge failures.
- 1994 Georgia storm: Over 500 bridges damaged





TxDOT bridge inventory

- 58,923 total bridges
 - 46,464 bridges over water (79%)
 - Of these:
 - 21,346 bridge class culverts (36%)
 - ~1% scour critical





Objectives of scour evaluation program

 Minimizing future flood damage requires greater focus on developing and applying improved procedures for designing and inspecting bridges for scour.



Importance of Scour Evaluation

- Scour documentation
 - Rapid evaluation in flood response
 - Prioritization of structures for replacement
 - Identify structures requiring repair
- Supports cost-effective maintenance and rehabilitation strategies.
- Helps prevent bridge collapse and loss of life.
- FHWA mandates evaluation of scour for all bridges over waterways.







Scour Documentation

Scour Summary Sheet supported by

- Scour analysis
- Scour screening evaluation
- Scour vulnerability assessment
- Risk assessment for unknown foundations
- Plan of action for scour critical structures
- In depth capacity analysis
- Plan of action follow up



Scour Summary Sheet Contents

- Bridge information
- Engineer's seal and signature
- Countermeasure condition
- Maximum allowable scour depth
- Observed scour depth
- Trigger elevation (for reevaluation)
- Description for future action

Scour depths are measured from:		
Abutment or Bent #	Bent 2	Abut 1
y _{ab} 🖌 or y _{ar}	15	
y _{al}	25	
Max Allowable Scour Depth ¹ , ya	15 (Elev. 200)	
Max Possible Scour Depth ²		See Trigge
Calculated Contraction Scour	5	0
Calculated Pier Scour	2.5	
Total Calculated Scour Depth	7.5	
Observed Scour Depth	10 (Elev. 195)	See below
Notes: (1) Min (yar or yab or yal). (2) ONLY a	oplicable if a non-ero	dible stratum

TRIGGER ELEVATION & FUTURE ACTION

Refer to Chapter 10 of the Scour Evaluation Guide.

Form 2605

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Current scour at abutment exposed toe wall of CRR. Considered moderate exposure condition. Re-evaluation will be needed: -When scour exposed the bottom of abutment cap -When scour at Bent 2 exceeded 13 feet (Elev. 198')



Maximum Allowable Scour – Span Bridges

- Amount of scour that can occur before a bridge foundation becomes unstable due to:
 - Bearing capacity
 - Lateral support
 - Rotational stiffness
 - Other applicable failure modes





Maximum Allowable Scour – Deep Foundations

- Geotechnical capacity assumptions
 - Factor of safety of 2 from original embedment
 - Uniform material along full length of element
 - End-bearing neglected
 - No disregard depth from original channel profile





Exposure Categories - Deep Foundations



*Note: When as-built Channel Profile is below the bottom of footing, Hr=0

1 Minor Foundation Exposure

3 Major Foundation Exposure

2 Moderate Foundation Exposure

(4) Extreme Foundation Exposure

y_a=Max Allowable Scour Depth (Refer to Ch. 2 in the Scour Evaluation Guide)



Exposure Categories - Abutments





Exposure Categories - Abutments







Exposure Categories - Abutments







Scour Analysis Methods

- Contraction Scour
 - Traditional HEC-18 method
 - Sandy Soils
 - SRICOS method
 - Clay and soft rock
 - Pressure method
 - Water above bridge low chord

- Pier Scour
 - Traditional HEC-18 method
 - Sandy Soils
 - SRICOS method
 - Clay and soft rock
 - Annandale's Erodibility Index method
 - Fractured/jointed rock



New Structures

- Scour Analysis based on hydraulic and hydrologic analysis **required** for all bridges
- Bridges designed to resist damage resulting from the scour design flood











Plan of Action Contents

- Bridge information
- Engineer's seal and signature
- Current scour and channel coding
- Scour vulnerability rating
- Monitoring plan
- Countermeasure recommendations



Plan of Action

Required for scour critical bridges

 Monitoring program considered as scour countermeasure only if specific details are provided (e.g., flood elevation or precipitation)

✓ Other Monitoring Program		
Type: 🖌 Visual 🗌 Other:		
Flood Monitoring Required: 🖌 Yes	No	
Trigger Conditions for Flood Monitoring:	Discharge:	
	✓ Stage:	50 yr flood elevation 182 ft
	Other:	



NOAA ATLAS 14

NOAA ATLAS 14 data can be used for the precipitation estimate

_	_	PUS-Gased	precipitation	u mednency	esomates v	NON WORK COM	moence me	evera (in inc	week.	
-		1.1		- 10	No. of Concession, Name	10	- 100		100	1000
1-100	6.378 (1.256-0-499)	0.644	6.358 (8-4)4-8 (102)	0.648	8.798 (0.557-1.84)	6.856	4.843 (0.658-1.36)	143	1.54	1.33
+	8.667 (1.465-0.562)	6.7%4 (0.540-0.541)	1.000	1.04 (8.786-1.30)	1,24	4.38 (1.965-1.90)	6.52 (1.05-2.17)	145 (119-241)	1.82	6.85
5-980	6.755 (5.575-6.996)	6.883 (0.675-1.96)	1.50 (0.041.1.40)	1.28	1.52 (0.10-2.07)	1.89 (1.30-2.38)	1.00	2.43 (1.56-2.96)	2,25 (1.46-3.37)	2.40
-	1.04 (8.786-1.37)	1,01	1.51	110110	2.87 (199-2.82)	2.58 (1.65-5.21)	2.53 (175-142)	2.76	3.07(139-6.01)	2,09
	8.30 (1.01-1.76)	1.56 (1.26-2.66)	1.96 (1.49-) 580	2,37 (1.25-3.482)	2.76 (196-147)	3.05 (2.15-4.18)	1.33 (2.154.34)	3.64 (2.45-5.24)	4.06	4.38
24	1.00	1.04 (1.47.2.47)	2.41	2.85	3.41	145	430	4.77 (3.25-6.89)	5.40 (110-5.94)	07848
34	1.36.2.37)	2.11	2.65 (2.65-3.48)	3.18 (2.424.16)	3.86	4.39	4.95	5.M (3.76.7.90)	6.38 (4.15.8.36)	1.04
-	2.80 (1.57-2.41)	3.48 (192-3.15)	2.18	2,79	4.87	6.38 (149.7.28)	4.11	6.NJ (#75-8.78)	6.15 H 25	575-13
фњ.	2.34 (1.45-2.94)	2.88	1.76 (2.96-640)	(3.44.5.70)	6.51 (#157.24)	6.38 (4.64-5.57)	7.33 (5 15 16 10	4.38 (5.16-11.7)	8.89 (8.16-14.2)	218-16
26.84	2.79	3.33	4,27	5.52	6.38	2.40 (5.45.9.57)	8.57 (8 71 71 16)	\$45 (842-0.6)	41.J (725-16.8)	452
2-600	3.43	383	4.89	5.85 (4.05.7.36)	7.25	8.42 (\$10 mil)	4.71 #.Mi-11-0;	46.3 (7.16-162)	43.3 (# 80 Mill)	85.8 (\$10.21
5-04	3.41 (2.73.4.34)	4.96 (3.2+4.89)	5.30	6.32	1.82 (1.86-1.88)	\$34 (\$72-11.8)	18.4 (7.18-13.6)	15.5	94.2 (0.0+10.0)	16.0
	3.62	4.40	1.41 (412-638)	6.08 (5.31.8.32)	8.M #30.910	8.52 (7.16.10.4)	11.9 (7 16.14.0)	12.5 (8.1716.8)	14.8 (9.97-29.3)	16.6
T-day	4.05	4.82	6.28 (5.16-7.86)	7.46	8.57	96.5 (7 86-19.5)	12.8 (174-15-7)	15.8	41.8 (10.0-21.0)	87.3 cm 7-34
5-m	4.45	6.34 (4.36-6-42)	6.94.4.20	1.06	9.86 (* 68.12.0	98.5 (835-56.6)	12.8	\$4.5 (10.2.19.1)	96.7 (11 4.22%)	16.5
ti-day	5.52 (4.13.4.46)	6.57 (5.65730)	8.29 a.m.+m	1.74 (7.85-11.8)	15.7	152	14.7	16.3 (18.7-21.5)	10.0	28.4
10-sity	6.45	F.56 (6.35.8 %)	1.48	11.8 (1.86-13.3)	10.1 (10.2-16.0)	14.8 (11.1-16.2)	18.J (115.21.0)	17.8	185 (115-26.0)	21.8
6-m	7.75 (8-45-9.20)	0.82 (7.61-10.0)	H.2 (675-10.2)	(118-11.5)	15.3	98.8 (12:8-29-8)	10.1.21.21	10.3 (14.5-25.8)	385 (11-6-29-4)	24.2 (76.3-52
ii day	14.116	16.3 (812-02.5)	42.7 (364-65.8)	54.5 (12.9-17.4)	47.8 (15.5.20.0)	18.8 (14.4.20.2)	28.6 (0.5.21.0)	22.3 (10.1-20.5)	34.7 (17.5.52.1)	26.4

✓ Other Monitoring Pro	gram		
Type: 🖌 Visua	Other:		
Flood Monitoring Rec	juired: 🖌 Yes	No	
Trigger Conditions fo	r Flood Monitoring:	Discharge:	
		✓ Stage:	2 yr 24 hr precipitation 3.0 inch
		Other:	



Example







Example (cont.)







Example (cont.)

 10.100	PR.0.41	0.07	
 111004		00	

The foundation is protected by a non-erodible stratum. (Describe below.)

Majority of DS is embedded in clays. Drilled-and-underreamed portion embedded in shale (range elev. 288 to 290').

Refer to Chapter 7 of the TxDOT Scour Evaluation Guide.

The foundation is supported by unknown foundations. (List any assumptions below.)

Since drilled-and-underreamed shafts were used and they were founded in shale, assumed end bearing has been considered as a major component of the bearing capacity.

Refer to Chapter 6 of the TxDOT Scour Evaluation Guide.

INSPECTION DETAILS

Date of Most Recent Inspection:

Scour countermeasures have been installed and are performing well. (Describe below.)

Condition of gabion baskets is still good (with a few baskets broken). Continue to monitor the gabion performance.

SCOUR DEPTHS

Scour depths are measured from the as-built channel profile.

Scour depths are measured from: Yal measured from nearest tie-beam (which vary)

Abutment or Bent #	4	5	6	7	
Yab 🖉 or y _{ar}	26' (elev 296')*	20' (elev 296')*	28' (elev 296')*	34' (elev 296')*	
Yal	34'	29'	34'	36'	
Max Allowable Scour Depth ¹ , y _a	26' (elev 296')*	20" (elev 296')*	28' (elev 296')*	34' (elev 296')*	
Max Possible Scour Depth ²					
Calculated Contraction Scour					
Calculated Pier Scour					
Total Calculated Scour Depth					
Observed Scour Depth	0.0	0.0	9.7	2.0	



Case Study (Cont.)

TRIGGER ELEVATION & FUTURE ACTION

Refer to Chapter 10 of the Scour Evaluation Guide.

*See calculation for deepening Yab based on end bearing and the strata of shale. Channel seems stable at elev 316' (or no vertical scour further), and has sign of later migration southward. Trigger for Bents 4 to 7: when observed scour reach elev. 296'.

Design load from as-built - For 24' roadway, 30.4 tons per shaft.	
Reving 18 - 600 - 600 - 703	
GENERAL NOTES: Design: H-15-44 Loading (Two lanes) in accordance in the 1- 1948 Specifications as amenaed by THD Supplement N. 11 All concrete shall be Class A. Chamfer all exceed concerns except as noted Dimensions relating to reinforcing steel are to centers of p Average Colocial 460 pile Koadis are: 21 Roadinay=218 Tons, 24 Poodinay 200 10 ns, 27 Ricadis are: 21 Roadinay=218 Tons,	*
Siec i forms as per included form defail sheet shall be used for barsts the for influst sina be default new industriante and st Siec iss-dif before heing weak to the rendering per	
30'-0" CONCRETE SLAB	
AND GIRDER SPAN	
21',24', 8 27' ROADWAYS - NO CURBS	5
H- 15 LOADING CG-	!
Sheet / of A	



Example (cont.)



Conservatively assumed the Shale is softer than 100 blows/12", see below Fig 5-2 in 2020 TxDOT Geotechnical Manual.



Conclusion

Once again,

- Scour documentation
 - Rapid evaluation in flood response
 - Prioritization of structures for replacement
 - Identify structures requiring repair
- Supports cost-effective maintenance and rehabilitation strategies.



Thank you!

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