

# **TXDOT PROJECT NO. 0-7193**

## ***DEVELOP ASSESSMENT AND MITIGATION GUIDANCE FOR ANCILLARY HIGHWAY STRUCTURES WITH EXISTING CRACKS***



*TASIG Meeting*  
**RESEARCH TEAM**

UT Austin – Junghoon Sohn, Shouchen Zhang (PhD Student), Dr. Mojtaba Aliasghar, Dr. Aidan Bjelland, Dr. Todd Helwig, Dr. Matthew Hebdon, Dr. Salvatore Salamone

Texas A&M – Hangil Kim (PhD Student), Dr. Arash Rockey, Dr. Stefan Hurlebaus, Dr. Peter Keating, Dr. Kinsey Skillen

**AUGUST 14, 2025**

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## **Research Team (RT) – UT Austin**

- Research Supervisors – Co-PIs: Todd Helwig, Matthew Hebdon, and Salvatore Salamone
- Graduate Research Assistants (GRAs and Post-Doctoral Researchers):
  - Junghoon Sohn and Shouchen Zhang (PhD Candidate)
  - Post-Doctoral Researchers: Mojtaba Aliasghar and Aidan Bjelland

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## Research Team (RT) – Texas A&M

- Research Supervisors – Co-PIs: Stefan Hurlebaus, Peter Keating, Kinsey Skillen, and Arash Rockey
- Graduate Research Assistants (GRAs):
  - HanGil Kim (PhD Candidate)

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## Presentation Outline

- Introduction and Background – Junghoon
- Field Assessment – Junghoon
- Lab Test Setup – Junghoon
- Current Results
  - UT – Shouchen
  - Texas A&M – Hangil
- Conclusion and Future work – Hangil

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## Introduction

- Ancillary structures experience repetitive fatigue loading
- Early detection is critical to prevent failure and reduce cost



Traffic Signal Structure  
(TSS)



Cantilever Overhead Sign Structures  
(COSS)



High Mast Illumination Pole  
(HMIP)

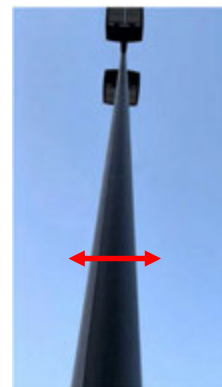
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## Fatigue Sources by Structure Type

- TSS/COSS: Vibration from mast arm (wind/galloping)
- HMIP: Wind-induced vortex shedding



Mast arm vibration (TSS/COSS)



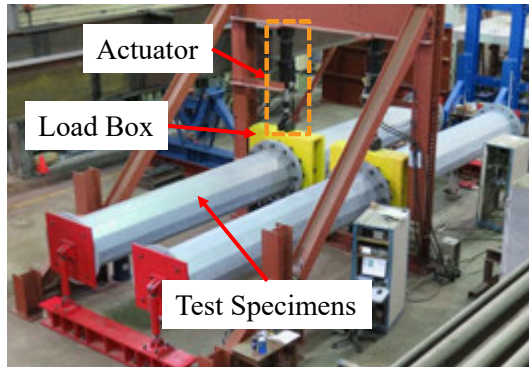
Wind-induced vortex shedding (similar to HMIP)

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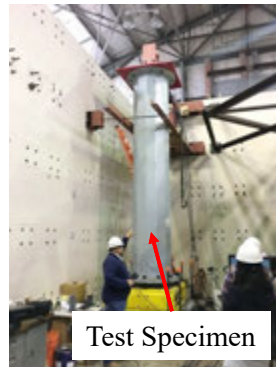


## Lab Testing Background

- Lab Test (Pool – 2010, Balivanis – 2013, Morovat et. al, 2018)
- Crack started at weld toe between baseplate and pole



Horizontal test setup



Vertical test setup



Test Results

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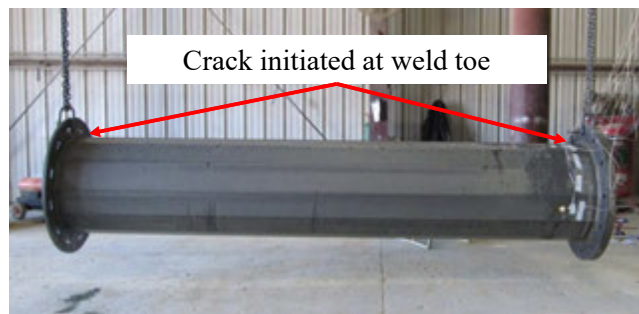


## Galvanizing-Induced Cracking

- Galvanizing monitoring (Kleineck – 2011)
- Thermal stress during galvanizing can cause galvanizing crack
- Cracks typically start at weld toe due to stress concentration



Thermal stress during galvanizing

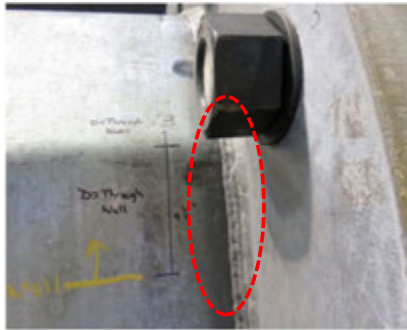


Typical location of galvanizing crack

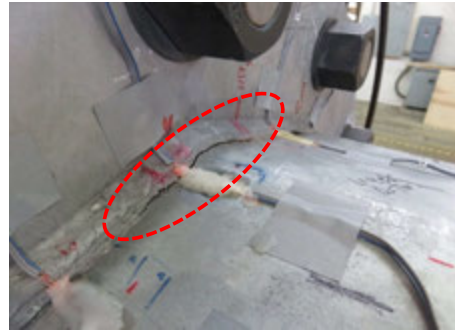
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## Challenge of Visual Inspection

- Cracks often remain invisible in early stages
- NDT (Non-Destructive Testing) is essential for early detection



Cracks not visible to eye

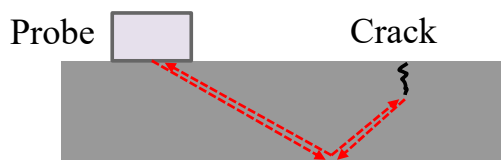
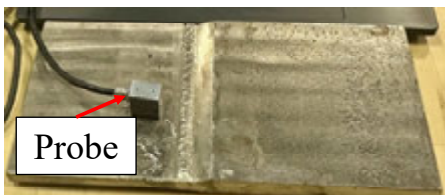


Visible significant crack

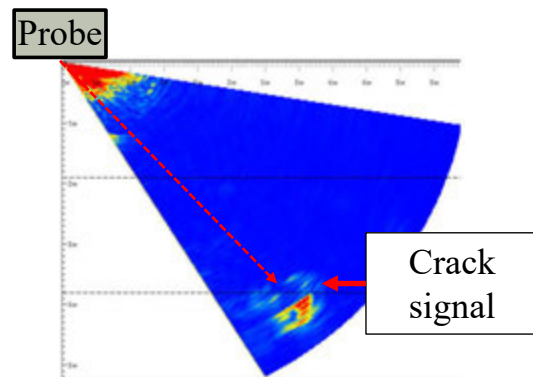
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## Challenge of Visual Inspection

- PAUT (Phased Array Ultrasonic Testing)
- Enables visualization of crack location and size



Crack monitoring using PAUT



PAUT result image

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## Crack Repair Method Overview

- Identify and fully remove the crack through grinding
- Restore the grinded area by rewelding the section



Crack identification



Crack removal (grinding)



Repair by rewelding

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## Research Objectives

- Field assessment using PAUT various jurisdictions of Texas
- Lab testing of repair strategies
- Provide practical recommendations
  - Monitoring
  - Repairing
  - Potential Component Replacement

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## Presentation Outline

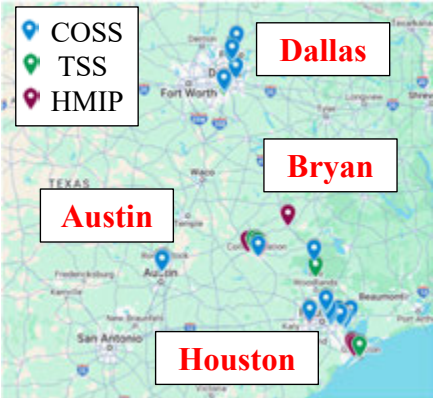
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## Field Assessment Scope

- 120 poles across Austin, Bryan, Dallas, Houston
- 40 each: COSS/ TSS/ HMIP



Structural Type	No. of inspected poles				Total
	Austin	Bryan	Dallas	Houston	
COSS	10	2	12	18	40
TSS	10	10	10	10	40
HMIP	10	10	10	10	40
Total					120

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## Field Assessment Procedure

- Measured dimensions and PAUT were used to detect cracks
- PAUT was effective in identifying weld toe cracks



Field assessment



Dimension Measurement



PAUT crack detection

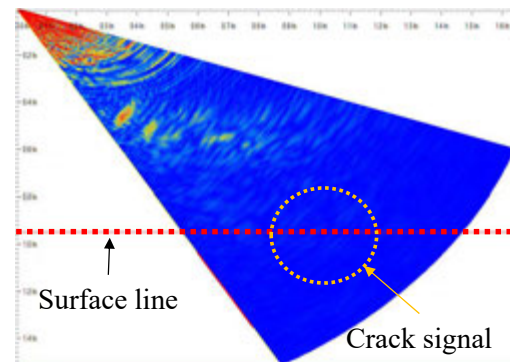
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## Field Assessment Procedure

- Measured dimensions and PAUT were used to detect cracks
- PAUT was effective in identifying weld toe cracks



PAUT during field assessment



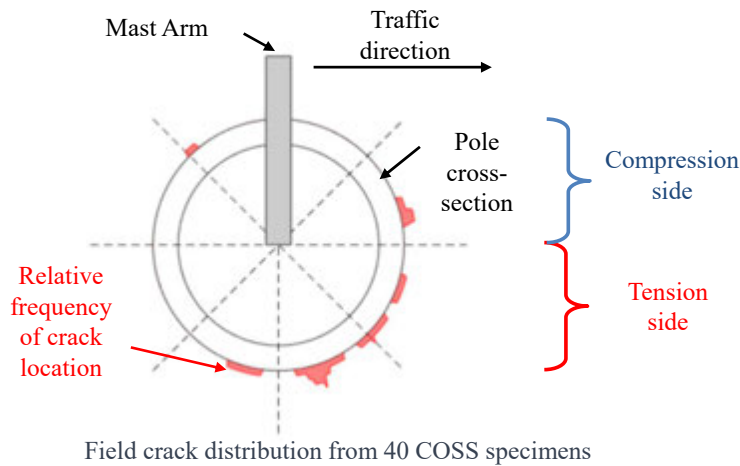
PAUT result

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## COSS Field Results

- Cracks primarily on tension side (opposite mast arm)

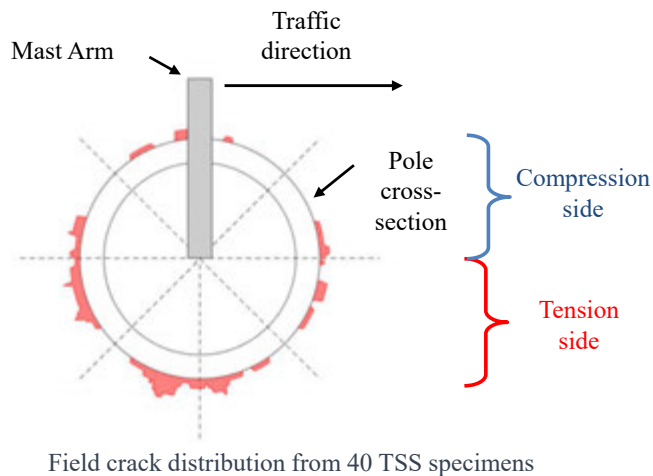


Common crack location of COSS

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## TSS Field Results

- Cracks more scattered, but concentrated on tension side



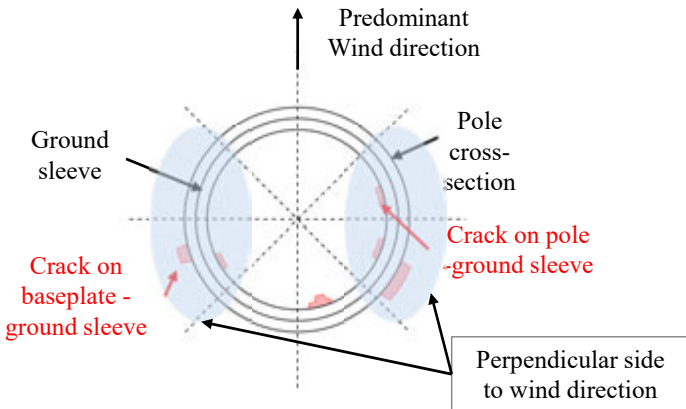
Common crack location of TSS

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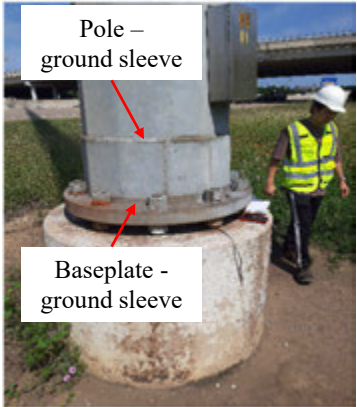


## HMIP Field Results

- Cracks primarily perpendicular to wind (vortex shedding)





The diagram shows a cross-section of a pole with a ground sleeve. A vertical arrow indicates the 'Predominant Wind direction'. Red arrows point to 'Crack on baseplate - ground sleeve' and 'Crack on pole - ground sleeve'. A box labeled 'Perpendicular side to wind direction' points to the crack locations. The text 'Field crack distribution from 40 HMIP specimens' is at the bottom.



The photograph shows a worker in a yellow vest and hard hat standing next to a large concrete baseplate. Red arrows point to the 'Pole - ground sleeve' and 'Baseplate - ground sleeve' areas. The text 'Common crack location of HMIP' is at the bottom.

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## Field Assessment Findings

- TSS & COSS: Cracks mainly located on the tensile side
- HMIP: Cracks typically occur perpendicular to the predominant wind direction
- Crack-prone locations should be regularly monitored based on structure type

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## Presentation Outline

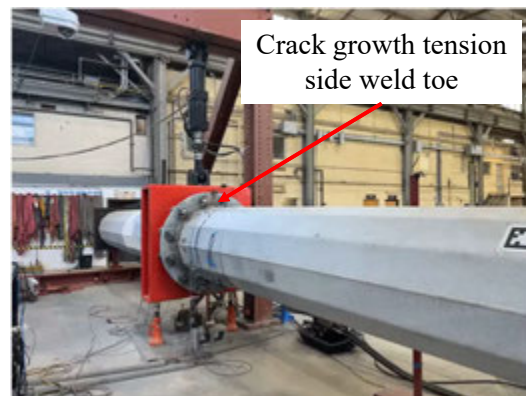
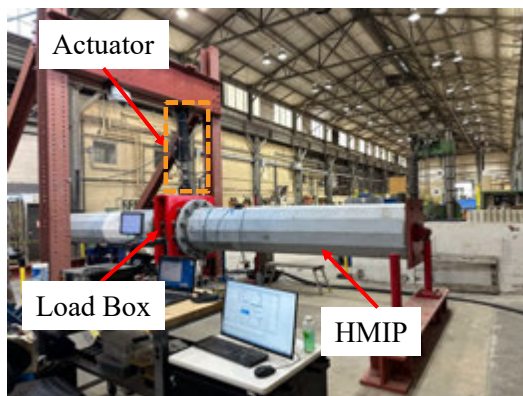
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## Lab Test setup

- UT: testing HMIP, COSS / A&M: testing TSS
- Actuator applies cyclic load through load box – upward only



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## Lab Monitoring & Repair

- Periodic PAUT scans monitor crack growth
- Repair triggered when crack exceeds length or depth threshold



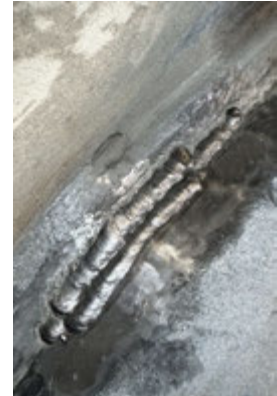
Crack monitoring (PAUT)



Ground area before rewelding



Rewelding in progress



Post-repair weld

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## Pre-Rewelding Strategy

- Option 1: Grind groove to remove crack
- Option 2: No groove, allow weld to penetrate crack



Crack removed prior to rewelding



Crack left in place prior to rewelding

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## Post-Rewelding Strategy

- Option 1: Leave weld as-is
- Option 2: Shallow weld toe grinding (<1mm depth)
- Option 3: Full surface sanding to remove surface irregularities



As-welded surface



Weld toe after shallow grinding



Smoothed surface after sanding

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## HMIP Test

- Specimens were taken from in-service HMIPs in Houston.
- Total length: 14 ft 4 in
- Wall thickness: 0.45 in



HMIP specimens transported from Houston to UT Austin



HMIP test setup

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## HMIP Test

<b>Geometry</b>	12-sided
<b>Diameter of base plate</b>	49 in.
<b>Wall thickness</b>	0.45 in.



HMIP test setup

Lab test setup			Field assessments (TXDOT Project 0-6829)		
Stress range	Controlling amplitude	Frequency	Effective stress range	Amplitude at 14'-4"	Cycles per day
7 ksi	0.19 in	1.25 Hz	0.6 ksi	0.01 in	26,500

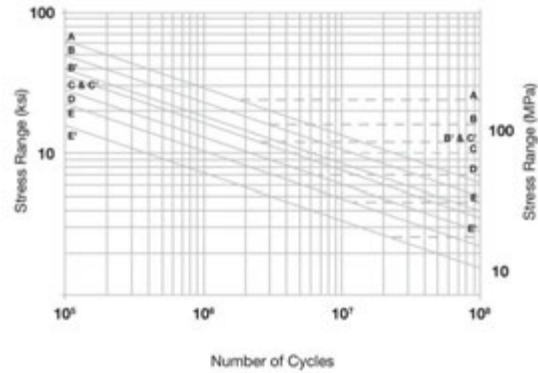
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## Conversion of Laboratory Data to Time Estimates in Field

S-N curve: 
$$\frac{N_{lab}}{N_{Field}} = \frac{S_{R_{field}}^3}{S_{R_{lab}}^3} \quad (1)$$

$$\Rightarrow N_{Field} = N_{lab} \cdot \left( \frac{S_{R_{lab}}}{S_{R_{field}}} \right)^3 \quad (2)$$

Lab test		Field (Project 0-6829)	
Stress range	Inspection interval	Effective stress range	Cycles per day
7 ksi	10,000	0.6 ksi	26,500

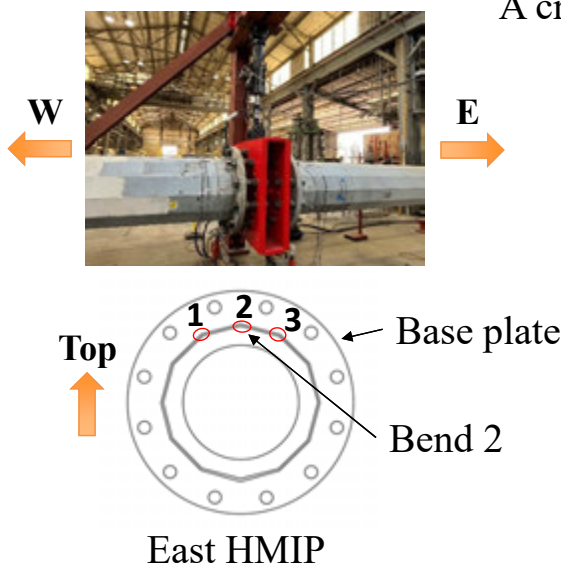


AASHTO Structural Supports Spec.: C11.9.3-1:  
Stress range vs. number of cycles

Applying 7 ksi to the HMIP in the lab for 10,000 cycles (2.5 hrs) is equivalent to approximately 1.6 years of natural wind loading in Austin.

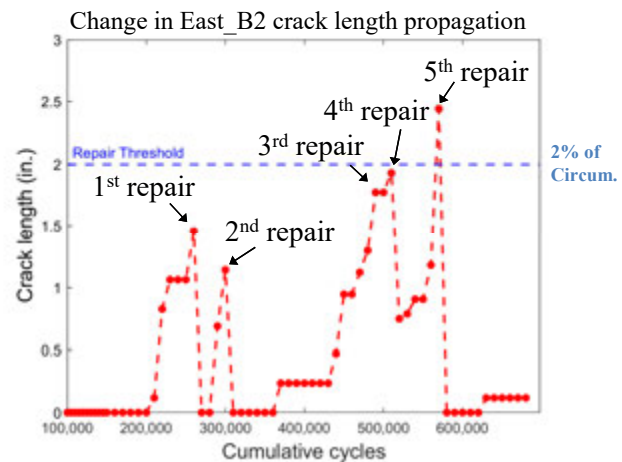
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## HMIP Test Results

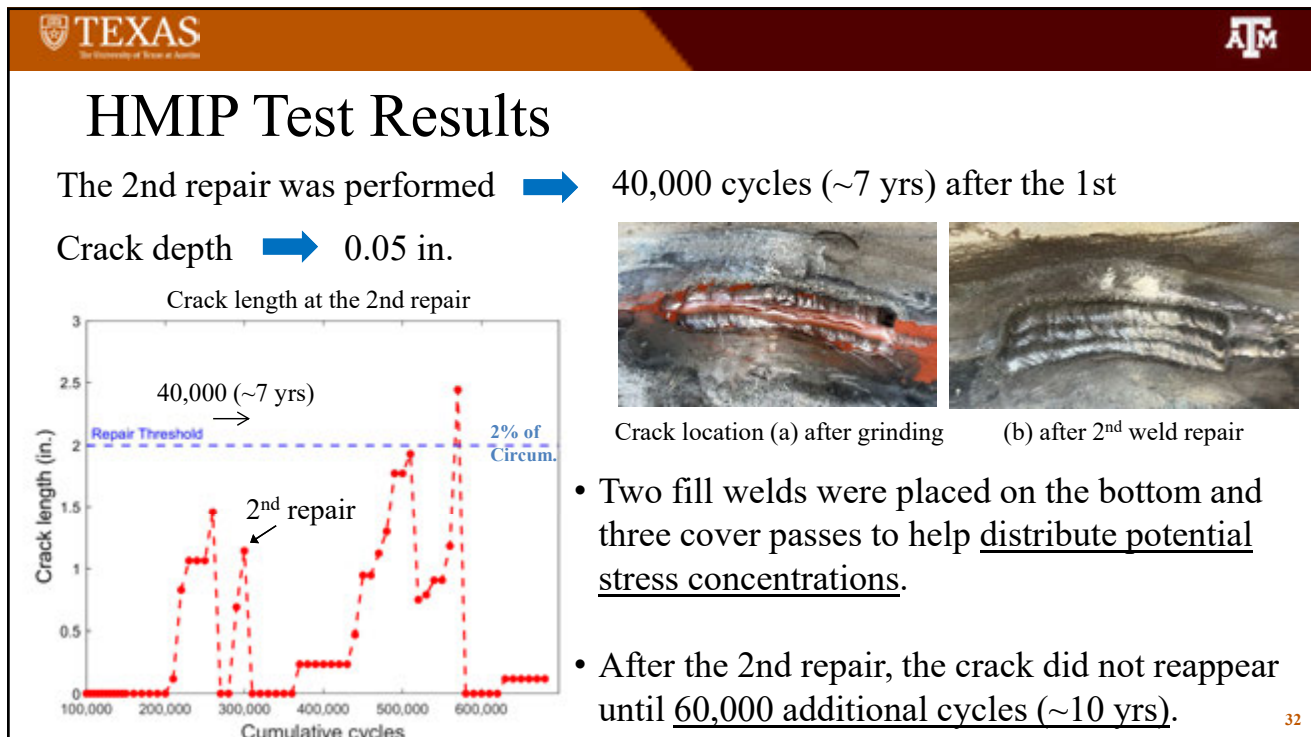
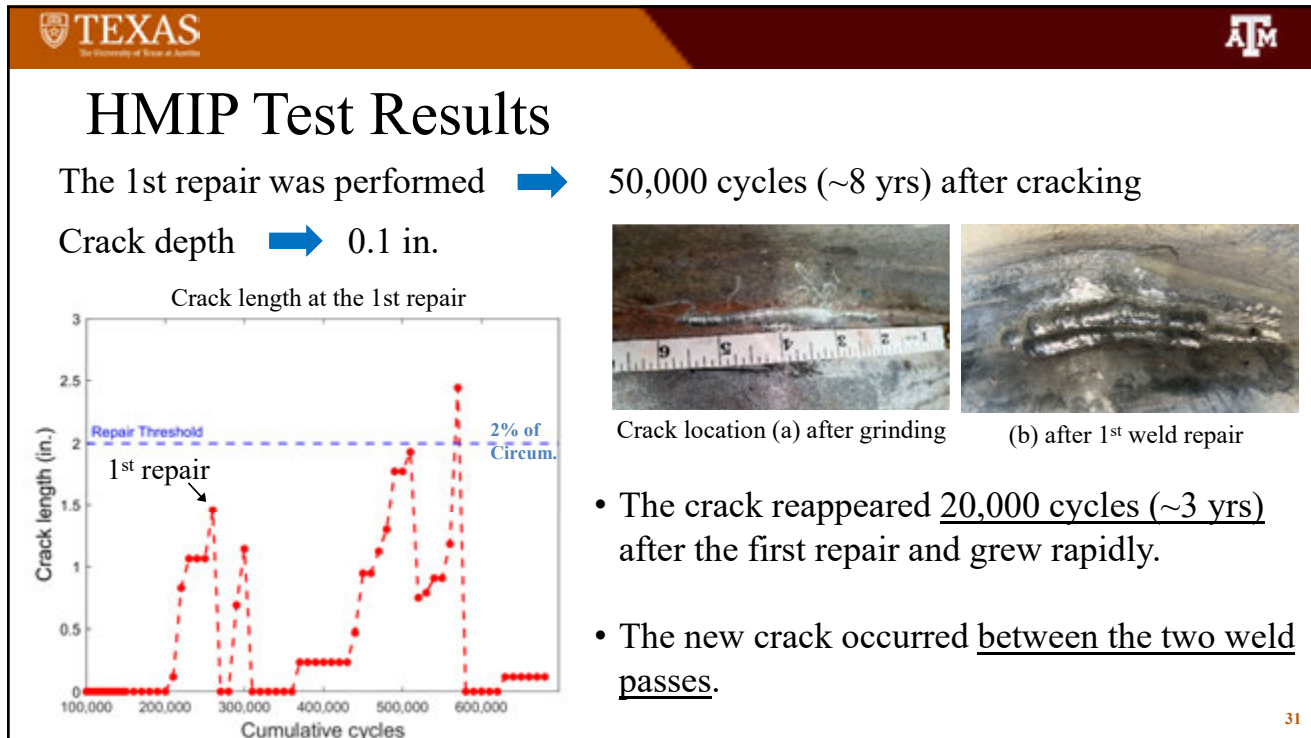


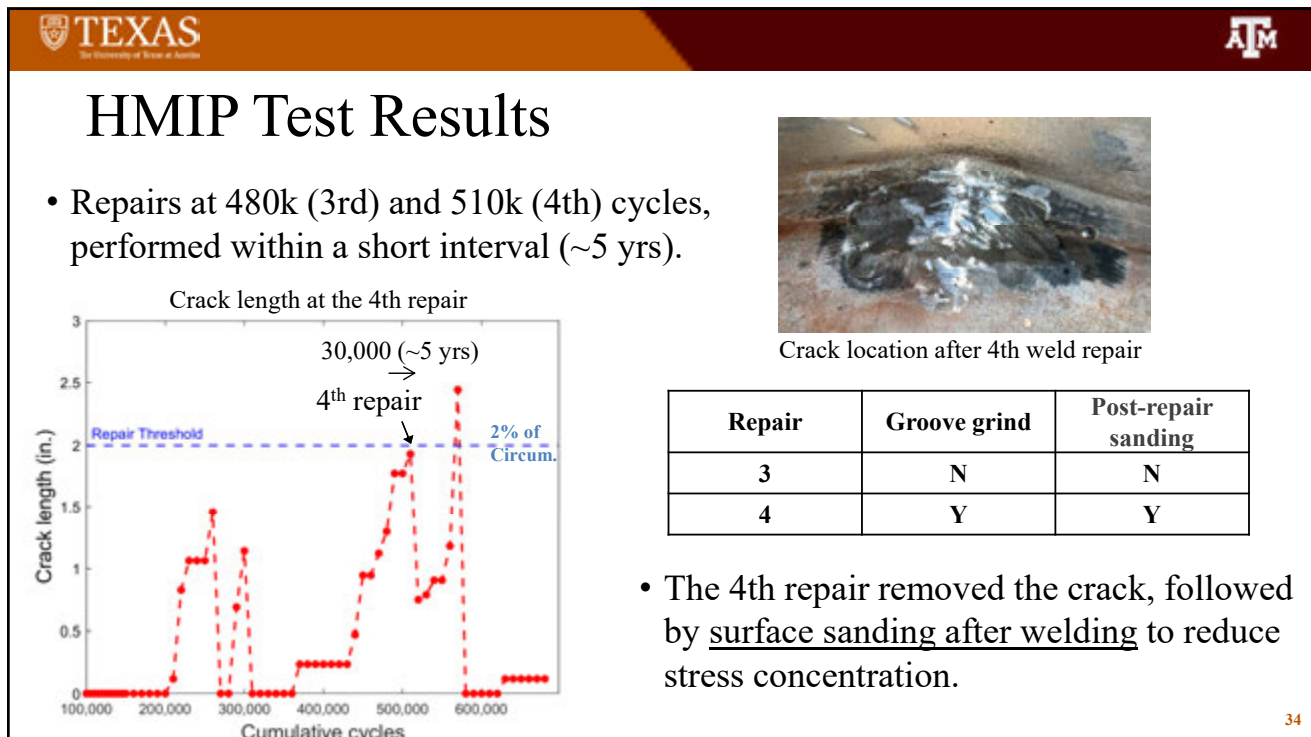
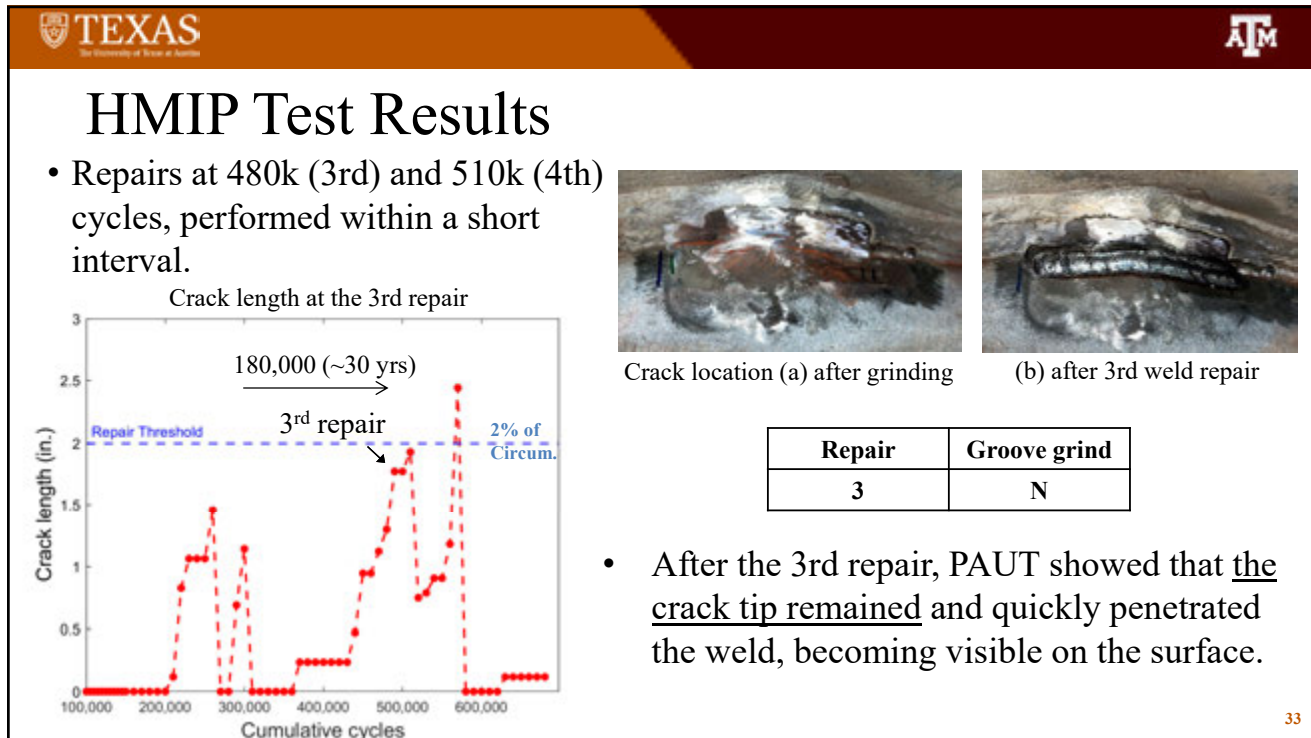
A crack first appeared → 210,000 cycles (approx. 34 yrs)

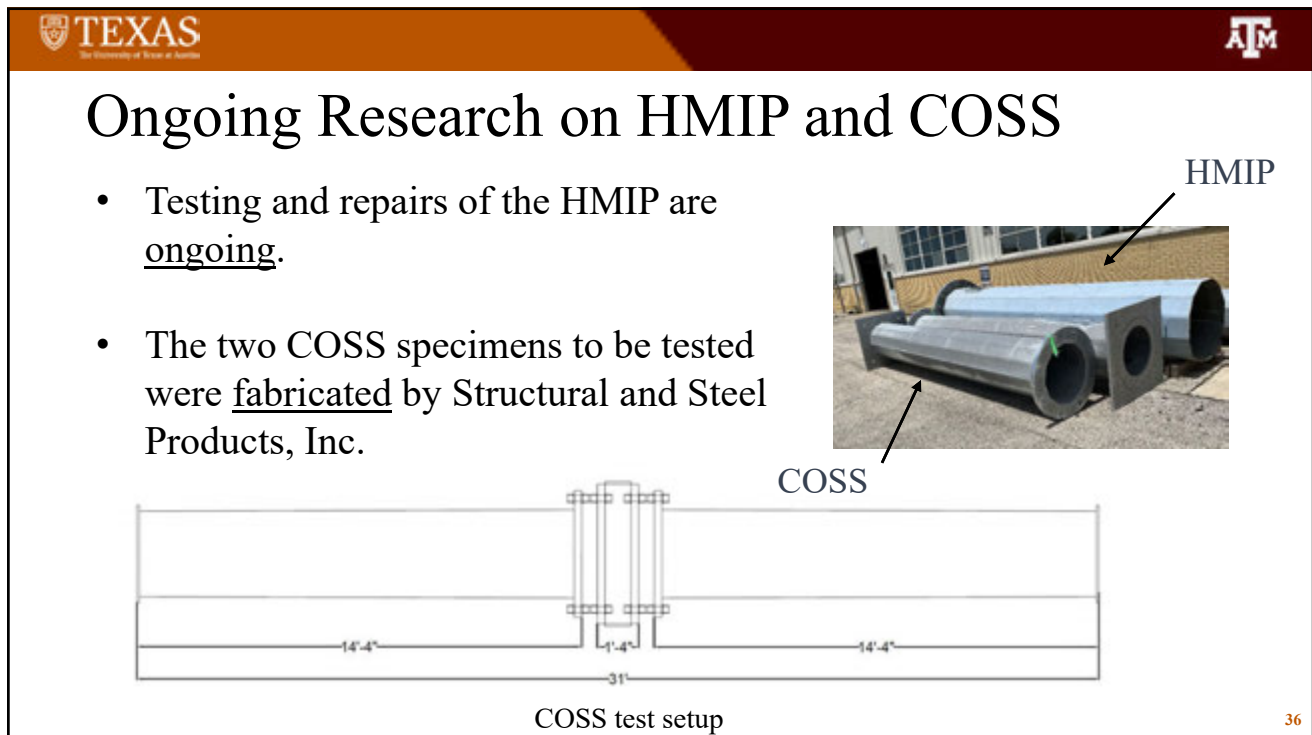
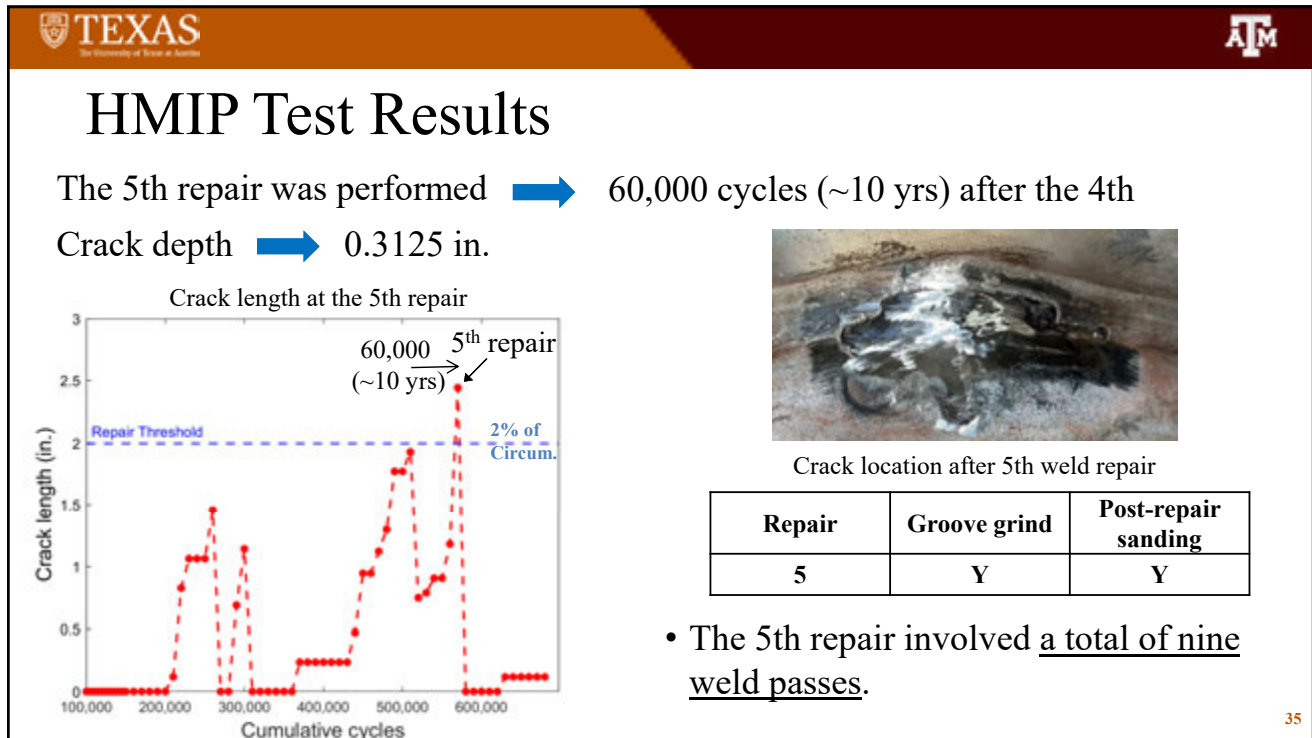
Welding repairs → 5 times



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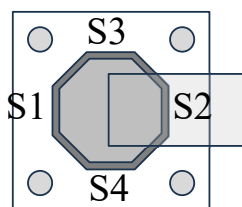
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## TSS Test Results



Location	West	East
Geometry	8-sided	Round
Diameter	12.0 in.	12.5 in.
Thickness	0.255 in.	0.181 in.

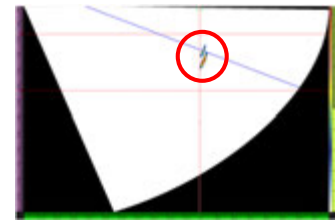
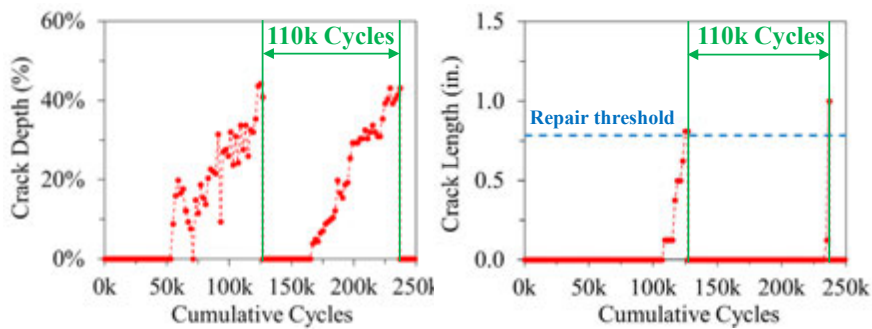


Setup	Stress Range	Loading Stress	Frequency
Side 1 (S1)	15 ksi	2 – 17 ksi	1 Hz
Sides 2–4 (S2–S4)	10 ksi	2 – 12 ksi	1.25 Hz

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## TSS Test Results

- Inspection interval: 2,000 cycles
- Inspection method: Visual inspection, PAUT
- Repair threshold: 2% circumference



PAUT result (S-Scan)

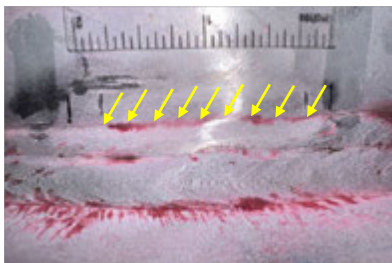


Crack configuration

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## TSS Test Results

- Repair procedure



Dye penetration test



Grinding and grooving

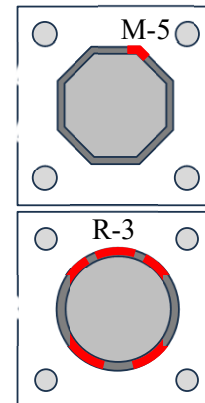
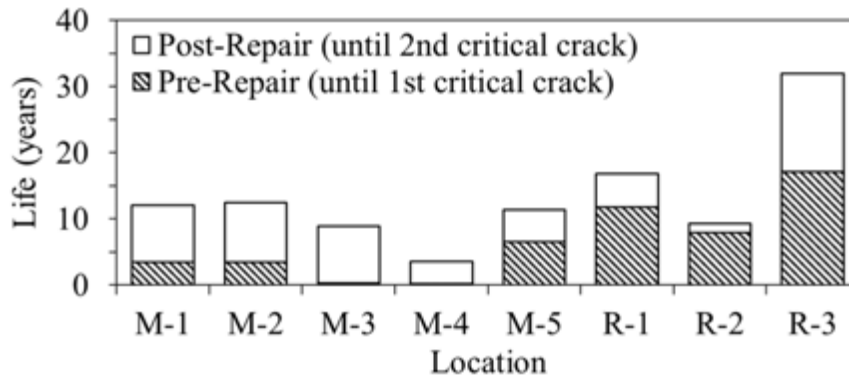


Welding

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## TSS Test Results

- Field pole (previously in service)
- Extended life after repair



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## TSS Test Results

2 Round Specimens – Fabricated by Valmont Industries

- Test plan: Fabricated round poles
- Outer diameter: 21 in.
- Wall thickness: 0.313 in.



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## Experimental Test Findings

- (Multi-Sided Pole) Fatigue crack initiates from bend
- Repairs → Extended fatigue life
- (TSS) Fatigue performance: Round Pole > Multi-sided pole

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## Conclusions

- TSS, COSS: Fatigue crack commonly occurs on the tension side (constant bending moment from mast arm dead load)
- HMIP: Fatigue crack commonly occurs perpendicular to the predominant wind direction (wind-induced vortex shedding)
- Multi-sided poles: Fatigue crack initiates from the bend
- Repair: Extend fatigue life of the pole

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## Future Work

- Complete experimental tests (TSS, COSS, HMIP)
- Develop certification methods for inspection personnel
- Finalize guidance on crack mitigation, repair, or replacement
  - Identify effective repair methods (e.g., grooving before, grinding after repair) and optimal repair timing (Crack depth and length)

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## Principle Investigator Contact Information

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# Thank you!

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